AD-756 198

THICKNESS REQUIREMENTS FOR SOILS BENEATH LANDING MATS; BARE BASE SUPPORT

Harry H. Ulery, Jr., et al

Army Engineer Waterways Experiment Station Vicksburg, Mississippi

January 1971

DISTRIBUTED BY:



Hational Technical Information Service U. S. DEPARTMENT OF COMMERCE 5285 Port Royal Road, Springfield Va. 22151



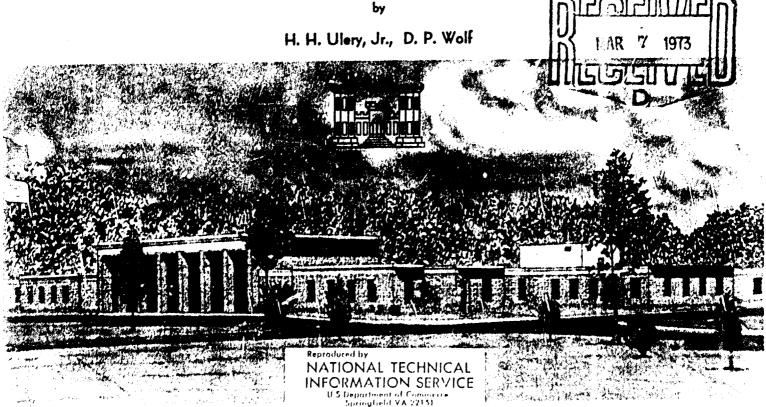
MISCELLANEOUS PAPER S-71-3

THICKNESS REQUIREMENTS FOR SOILS BENEATH LANDING MATS

BARE BASE SUPPORT

Project 3782-64





January 1971

Sponsored by Office, Chief of Engineers and U.S. Air Force

Conducted by U. S. Army Engineer Waterways Experiment Station, Vicksburg, Mississippi

Unclassified Security Classification

DOCUMENT CONTI							
(Security classification of title, body of abstract and indexing a 1. ORIGINATING ACTIVITY (Corporate author)	nnotation must be d		CURITY CLASSIFICATION				
, , ,			nclassified				
U. S. Army Engineer Waterways Experiment Sta Vicksburg, Mississippi	ation	2b. GROUP					
3. REPORT TITLE							
THICKNESS REQUIREMENTS FOR SOIL; BENEATH LAI	NDING MATS,	Bare Base	Support				
4. DESCRIPTIVE NOTES (Type of report and inclusive dates) Final report							
S. AUTHORIS) (First name, micillo initial, last name)							
Harry H. Ulery, Jr. Denis P. Wolf							
6. REPORT DATE	74. TOTAL NO. O		75. NO. OF REFS				
January 1971	125		5				
SA. CONTRACT OR GRANT NO.	M. ORIGINATOR	S REPORT NUMB	(EA(4)				
A. PROJECT NO. 3782-64	Mis	scellaneous	Paper S-71-3				
e.	98. OTHER REPORT HOIS) (Any other members that may be excigned this report)						
4	}						
10. DISTRIBUTION STATEMENT							
This document has been approved for public unlimited.							
11- SUPPLEMENTARY HOTES	12. EPONSCRING						
			of Engineers				
`		Air Forc					
		nington, D.					
ducted at the U. S. Army Engineer Waterways developing a method for determining thickne membrane-surfaced, and unsurfaced airfields this report pertains to the development of ments of soil strengthening layers for land mat-surfaced test sections were constructed tions consisted of heavy clay (1.3 to 3.7 C rial placed at a higher strength (3.0 to 8. tween the landing mat and subgrade. Test s test sections II, III, and IV were surfaced was surfaced with AM2 mat. Aircraft traffi 70,000 lb with tire pressures ranging from configurations spaced 32 in. center-to-cent lb and tire pressures ranging from l09 to 1 carts. Traffic was applied until each test density of the subgrade and overlying higheing, and after the traffic tests, and the c Deflections and deformations were determine	Experiment ss requirement. The phase a method for ing-mat-suri and tested. BR) of vario 0 CBR) was vection I was with XM18 I c with single 112 to 229 I er with loss 22 psi were section fair strength condition of	Station for the property of th	r the purpose of nding-mat-surfaced, ogram presented in ng thickness require-elds. Five landing-rades of the test secses. The same mate-trengthening layer bewith M&Al landing mat; and test section V ads of 25,000 to ffic with twin-wheel from 56,000 to 70,000 by means of test load water content, and measured before, durections was recorded.				

DD . 1473 SECLASES POR ARMY USE. JAH ON, WHICH IS

Unclassified

Ta

termining the required thickness of soil strengthening layers beneath landing mat was developed by correlating the data from this and previous studies with flexible pavement design relations. This equation is proposed for use in establishing design criteria for thicknesses of soil strengthening layers beneath landing mat.

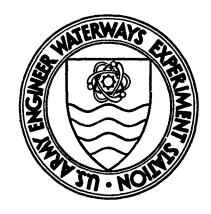
•

Unclassified

KEY WORDS	LIN	KA	LIN	K B	LINK C		
	ROLE	WT	ROLE	WT	HOLE	*	
irfields]					
sare base support	1	1					
anding mats	ľ	<u> </u>					
Soil strength	Į.						
Surfaced airfields]						
Insurfaced airfields							
				1			
	1	}		1			
]					
	-						
	j			l			
	İ	1					
		1					
		1					
	1						
		1					
					1		
				ĺ			
				Ì			
				i			
	Į						
			1	}			
	-						
	1]			Ì	
	}	}	1				
	į						
]						
	İ	1		1			
	1						
		1		ł		ľ	

Unclassified
Security Classification

ID



MISCELLANEOUS PAPER S-71-3

THICKNESS REQUIREMENTS FOR SOILS BENEATH LANDING MATS

BARE BASE SUPPORT

Project 3782-64

Ьу

H. H. Ulery, Jr., D. P. Wolf



January 1971

Sponsored by Office, Chief of Engineers and U. S. Air Force

Conducted by U. S. Army Engineer Waterways Experiment Station, Vicksburg, Mississippi

ARMY, MAC VICKERIJEG MISS.

71

This document has been approved for public release and sale; its distribution is unlimited

Foreword

The study reported herein was a combined study authorized by the Office, Chief of Engineers, in "Instructions and Outline for Development of Thickness Design Criteria for Landing-Mat- and Membrane-Surfaced and Unsurfaced Airfields, FY 1967," dated May 1966, and by U. S. Air Force (USAF) MIPR No. AS-7-333, dated 3 April 1967, under the general project title Bare Base Support. The study was conducted by personnel of the U. S. Army Engineer Waterways Experiment Station (WES), Vicksburg, Miss., during the period August 1966-March 1969.

General supervision of this study was conducted by Messrs. W. J. Turnbull, A. A. Maxwell, R. G. Ahlvin, D. N. Brown, and C. D. Burns of the Soils Division, WES. Personnel actively engaged in the planning, testing and analyzing phases of this study were Messrs. H. H. Ulery, Jr.; W. N. Brabston; D. M. Ladd; G. M. Hammitt II; J. E. Watkins; and D. P. Wolf. This report was prepared by Messrs. Ulery and Wolf.

Directors of the WES during the conduct of this study and the preparation of this report were COL John R. Oswalt, Jr., CE, COL Levi A. Brown, CE, and COL Ernest D. Peixotto, CE. Technical Directors were Messrs. J. B. Tiffany and F. R. Brown.

Contents

Pag	e
Forewordii	i
Conversion Factors, British to Metric Units of Measurement vi	i
Summary	x
Introduction	1
Objective	1
****	1
Descriptions of Test Sections and Load Vehicles	1
	1 2
Application of Traffic and Failure Criteria	4
::pp==================================	4 4
Data Collection	5
Soils data	5 5
Fraffic Test Results	5
Test section I	56 7 7
	7
	7
Conclusions	.1
Literature Cited	.1
Tables 1 and 2	
Plates 1-6	

Preceding page blank

Contents

																												rage
Appen	dix A	: Tr	aff:	ic :	Гe	sts	3.	•	•	•	•	•	•	•	٠	•	•	•	•	•	•	•	•	•		•	•	Al
	Test	Sect	ion	I					•			•							•				•		•			Al.
	Test	Sect	ion	II				•	•		•	•	•	•	•			•			•		•		•	•	•	A5
	Test	Sect	ion	II	I.			•	•			٠	•	•	•	•		•	•	•	•	•	•	•			•	8 A
	Test	Sect	ion	IV				•	•	•	•	•	•	٠	•	٠	•	•	•	•	•	•	•	•	•	•	•	A1.1
	Test	Sect	ion	V	•	•		•	•	•	•	•	•	•	•	•	•	•	•	•	•	•	•	•	•	•	•	A12
Table	s Al-	A 3																										
Photo	graph	s Al-	A77																									
Plate	s Al-	A40																										
Appen	dix B	: Th	ickı	nes	s l	Rec	luc	ti	.on	C	ur	ve	s	•	•	•	•	•	•	•	•	•	•	•	•	•	•	B 1
Plate	Bl																											
Appen	dix C	: De	sign	1 C	ur	ve	₃.	•	•	•	•		•	•	•	•			•	•	•		•		•	•	•	Cl
Plate	s Cl :	and C	2																									

Conversion Factors, British to Metric Units of Measurement

British units of measurement used in this report can be converted to metric units as follows:

Multiply	B y	To Obtain							
mils	0.0254	millimeters							
inches	2.54	centimeters							
feet	0.3048	meters							
square inches	6.4516	square centimeters							
pounds	0.45359237	kilograms							
kips	453.59237	kilograms							
pounds per square inch	0.070307	kilograms per square centimeter							
pounds per cubic foot	16.0185	kilograms per cubic meter							

Summary

The study reported herein is one phase of the research program being conducted at the U. S. Army Engineer Waterways Experiment Station for the purpose of developing a method for determining thickness requirements for landing-mat-surfaced, membrane-surfaced, and unsurfaced airfields. The phase of the program presented in this report pertains to the development of a method for determining thickness requirements of soil strengthening layers for landing-mat-surfaced airfields.

Five landing-mat-surfaced test sections were constructed and tested. The subgrades of the test sections consisted of heavy clay (1.3 to 3.7 CBR) of various thicknesses. The same material placed at a higher strength (3.0 to 8.0 CBR) was used as a strengthening layer between the landing mat and subgrade. Test section I was surfaced with M8Al landing mat; test sections II, III, and IV were surfaced with XMl8 landing mat; and test section V was surfaced with AM2 mat.

Aircraft traffic with single-wheel loads of 25,000 to 70,000 lb with tire pressures ranging from 112 to 229 psi and traffic with twin-wheel configurations spaced 32 in. center-to-center with loads ranging from 56,000 to 70,000 lb and tire pressures ranging from 109 to 182 psi were simulated by means of test load carts. Traffic was applied until each test section failed. CBR, water content, and density of the subgrade and overlying higher strength layer were measured before, during, and after the traffic tests, and the condition of the test sections was recorded. Deflections and deformations were determined throughout testing.

An equation for determining the required thickness of soil strengthening layers beneath landing mat was developed by correlating the data from this and previous studies with flexible pavement design relations. This equation is proposed for use in establishing design criteria for thicknesses of soil strengthening layers beneath landing mat.

Preceding page blank

THICKNESS REQUIREMENTS FOR SOILS BENEATH LANDING MAT

BARE BASE SUPPORT

Introduction

Background

1. In many areas of the world, the in situ soil does not have the strength required to support aircraft operations. This requires the placement of a stronger medium over the weak soil. This, at times, can be accomplished by placing (a) landing mat on the soil, (b) a layer of stronger soil on the weak soil, or (c) a combination of both. This investigation is a study of the latter condition conducted by the U. S. Army Engineer Waterways Experiment Station (WES) for the Office, Chief of Engineers, and the U. S. Air Force.

<u>Objective</u>

2. The overall objective of this study was to determine the minimum required thickness of soil having a strength at least equivalent to that required for a subgrade directly under landing mat by investigating the effects of load, tire pressure, and soil strength on the performance of landing mat under traffic.

Scope

3. The objective was accomplished by constructing five test sections and subjecting the sections to accelerated traffic using various single-and twin-wheel loadings and tire pressures. This report presents a description of the materials used, test sections, construction methods, tests conducted and results, and an analysis of the results. Related data obtained from previous studies were also used in the analysis.

Descriptions of Test Sections and Load Vehicles

Test sections

4. Five special test sections were constructed under shelter at WES in order that water content and strength of the subgrade could be

controlled. The test sections will be referred to as test sections I through V in this report.

5. The construction of the test sections was accomplished by excavating a specified area for each test section, backfilling the excavation in 6-in.* lifts with a heavy clay (CH),** and comparting each lift with a self-propelled rubber-tired roller. A brief description of each test section is presented as follows:

			Thick	ness, in.
Test Section	Type Mat	Item No.	Subgrade	Strengthening Layer
I	M8A1	1 2 3 4	30 24 16 - 12	6 12 20 24
II	хм1.8	1 2 3 4	28 23 18	7 12 17 35
III	XML8	1 2 3	28 22 17	6 12 17
ľV	XMI8		18	16
V	AM2		30	6

The subgrades of each of the test items consisted of low-strength clay (1.3 to 3.7 CBR) of various thicknesses. The same material with a higher strength (3.0 to 8.0 CBR) was used as a strengthening layer over the subgrade. In test sections III and IV, 6-mil-thick polyethylene was placed at the interface of the subgrade and the strengthening soil layer to facilitate deformation measurements of the subgrade after completion of traffic tests. Plan and profile views of test sections I through V are shown in plates 1-5, respectively.

Load vehicles

6. Two types of load vehicles were used in trafficking the test

^{*} A table of factors for converting British units of measurement to metric units is presented on page vii.

^{**} Classified according to reference 1.

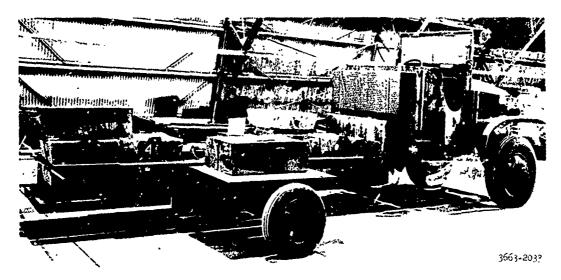


Fig. 1. Test vehicle for 25,000- to 30,000-1b loads

sections. A specially designed single-wheel load cart (fig. 1) was used for tracking with loads varying from 25,000 to 30,000 lb. It was equipped with an outrigger wheel to prevent overturning and was powered by the front half of a four-wheel-drive truck. The load vehicle shown in fig. 2 was

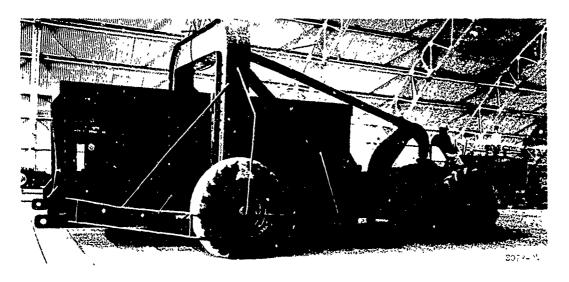


Fig. 2. Test vehicle for 31,000- to 75,000-lb loads

used for tracking with loads varying from 31,000 to 75,000 lb. It consisted of a box-type load compartment and was powered by a two-wheel tractor unit. The tracking wheels within the load compartment were

interchangeable, and single- and twin-wheel assemblies with various tire sizes were used for traffic tests. Pertinent tire data for the various test sections are summarized in table 1.

Application of Traffic and Failure Criteria

Application of traffic

- 7. Traffic was applied to the test sections to simulate the traffic distribution pattern that would be encountered in actual aircraft operation on a taxiway. Each test lane was trafficked by starting at one side of the test lane and driving the load cart forward and then backward in the same path for the length of the traffic lane. The path of the cart was then shifted laterally one tire print width on each successive trip, thus producing two coverages of the entire traffic lane when the load cart had maneuvered from one side of the traffic lane to the other. The number of passes made in each track was varied to provide 100 percent coverage on an area down the center of the traffic lane with less coverage along the edges. All data used for analysis were obtained from 100 percent coverage areas. Failure criteria
- The failure criteria used in these tests were based primarily on the development of roughness and excessive mat breakage due to subgrade deformation. When surface deviations from a 10-ft straightedge approached or equaled 3 in. in any direction within the traffic lane, the test item was considered failed due to roughness.
- 9. Failure due to mat breakage was based on sufficient breakage to represent a tire hazard during aircraft operations. It was assumed that a certain amount of maintenance would be performed in the field during actual usage and that minor metal or weld breaks could be easily repaired. It is considered feasible to replace up to 10 percent of the mat panels with new mat during the design service life of a runway; however, replacement in excess of 10 percent is considered excessive. Therefore, in these tests, it was assumed that up to 10 percent of the mat panels could be replaced, and when an additional 10 percent of the panels had failed (a total of 20 percent failed), the entire test item was considered failed.

Data Collection

worls data

10. Water content, dry density, ari in-place CBR tests were conducted on the strengthening soil layer and subgrade in each test item prior to traffic, at intervals during traffic, and at failure of each test item. These tests were conducted on the surface of the strengthening layer and at intermediate depths up to a depth of approximately 12 in. into the subgrade of each test item. A minimum of three determinations was made at each increment of depth, and, in general, the values reported herein are averages of the values ascertained at each particular depth.

Traffic data

11. Visual observations of the behavior of the test items under traffic and other pertinent data were recorded throughout the traffic test period. These observations and data were supplemented by photographs. Level readings were taken on the mat prior to traffic and at intervals throughout the traffic test period to record the development of permanent mat deformation and elastic deflection of the mat under the wheel load. Elastic deflections were measured at two locations, i.e., with the tire centered over the center of a panel and with the tire centered over an end joint. Roughness of the test items was determined at various intervals during the traffic test period by measuring the deviation of the mat surface from a 10-ft straightedge placed in longitudinal, transverse, and diagonal positions on the mat surface.

Traffic Test Results

12. All test items were trafficked to failure. Details of the traffic tests are given in Appendix A; the traffic tests are summarized below and in table 2.

Test section I

13. A plan and profile of test section I are shown in plate 1. The plan of lane 3 is shown separately in plate 1 for clarity. A 31,000-lb single-wheel load utilizing a 56x16 tire with an inflation pressure of

185 psi was used to traffic lane 1. Lane 2 was trafficked with a 56,000-lb twin-wheel load utilizing 56xl6 tires with an inflation pressure of 105 psi. After 612 coverages, the load was increased to 62,000 lb with an inflation pressure of 185 psi. Lane 3 was trafficked with a 62,000-lb twin-wheel load utilizing 56xl6 tires with an inflation pressure of 185 psi. Performance under traffic was as follows:

		Coverages		CBR
Test <u>Lane</u>	Item No.	at <u>Failure</u>	Subgrade	Strengthening Layer
1	1	30	2.3	6
	2	110	2.8	7
	3	310	3.2	8
	4	430	3.2	7
2	1	120	2.3	5
	2	612	2.8	6
	3	1091	2.9	6
	4	1324	2.8	7
3	2	120	2•3	6
	3	408	3•0	7
	4	750	3•0	7

Test section II

14. A plan and profile of test section II are shown in plate 2. A 30,000-lb single-wheel load utilizing a 30x11.5 tire with an inflation pressure of 250 psi was used to traffic lane 1. Lane 2 was trafficked with a 70,000-lb twin-wheel load utilizing 44x16 tires with an inflation pressure of 185 psi. The performance of the test section is summarized below.

		Coverages	CBR							
Test <u>Lane</u>	Item No.	at Failure	Subgrade	Strengthening Layer						
1	1 2 3 4	72 170 202 202	1.3 2.3 1.4	3.0 3.1 3.4 3.4						
2	1 2 3 4	32 60 144 300	1.4 1.7 1.7	3.3 3.1 3.4 3.7						

Test section III

15. A plan and profile of test section III are shown in plate 3. A 25,000-lb single-wheel load utilizing a 30xll.5 tire with an inflation pressure of 250 psi was used to traffic lane 1. Lane 2 was trafficked with a 75,000-lb single-wheel load utilizing a 25.00x28 tire with an inflation pressure of 125 psi. Test section performance under traffic was as follows:

		Coverages	CBR						
Test Lane	Item No.	at <u>Failure</u>	Subgrade	Strengthening Layer					
1	1	528	2.1	7					
	2	884	2.0	7					
2	1	56	2.1	8					
	2	72	1.8	7					
	3	92	1.9	6					

Test section IV

- 16. A plan and profile of test section IV are shown in plate 4. The test section consisted of only one test item and was trafficked with a 60,000-lb single-wheel load utilizing a 25.00x28 tire with an inflation pressure of 125 psi. The section failed after 348 coverages. The CBR's of the subgrade and strengthening layer were 1.8 and 7, respectively. Test section V
- 17. A plan and profile of test section V are shown in plate 5. The test section consisted of one test item and was surfaced with one-, two-, and three-piece AM2 landing mat and was trafficked with a 25,000-lb single-wheel load utilizing a 30x11.5 tire with an inflation pressure of 250 psi. The section failed after 330 coverages. The CBR's of the subgrade and the strengthening layer were 3.7 and 5, respectively.

Analysis, Conclusions, and Recommendations

Analysis

18. The method used to analyze the test data was to relate the load-carrying capabilities of the various types of landing mat to the load-carrying capabilities of a flexible airfield pavement. This was accomplished by expressing mat performance in terms of thickness (top of

subgrade to pavement surface) of conventional flexible airfield pavement effectively replaced by the landing mat (thickness reduction) if both were placed on the same strength subgrade.

19. Since this investigation included strengthening layers of various thicknesses between subgrade and landing mat, the effective thickness of this type of mat-soil system could be defined as the sum of the actual thickness of the strengthening layer and the thickness reduction for the type of mat being tested and analyzed. The reduction in thickness (which varies with load and tire pressure) of subbase, base, and pavement that can be applied to the pertinent flexible pavement thickness in establishing thickness requirements for landing mat can be obtained from reference 2 for M6 and M9 mat and from Part I of reference 3 for M8 mat. Thickness reduction criteria for M8A1, XM18, and AM2 landing mat used in this analysis were obtained from preliminary relationships developed from other studies and are shown in Appendix B.

20. By using the CBR equation shown below, a required thickness of flexible pavement structure can be calculated that provides the same load-support capability for each loading and subgrade condition found in the actual landing mat tests. This thickness can then be compared with the effective mat-soil thickness. The following equation was used to determine the total required flexible pavement thickness:

$$t = (0.23 \log C + 0.15) \sqrt{\frac{P}{8.1 \text{ CBB}} - \frac{A}{\pi}}$$
 (1)*

where

t = total thickness of flexible pavement structure (above subgrade), in.

C = number of coverages

P = single- or equivalent single-wheel load, 1b

CBR = measure of subgrade strength

A = tire contact area, sq in.

This is a combination of equation 2, page 2, and the equation for slope of curve, plate 6 in reference 4.

In the case of the twin-wheel configurations, a means of relating the twin loading to an equivalent single-wheel load is required, and the procedure outlined in reference 4 was used.

- 21. Equation 1 was also used to establish an equivalent number of coverages for test items subjected to mixed traffic. Items 3 and 4 of lane 2, test section I, were trafficked with both a 56- and 62-kip twin-wheel loading. In order to establish an equivalent 56-kip twin-wheel loading coverage level, the CBR equation was used to obtain a flexible pavement thickness, based on actual test conditions, for the 62-kip twin loading. This thickness was then used to determine the equivalent coverages of the 56-kip twin-wheel loading. This coverage value plus the actual coverages applied by the 56-kip twin loading represents the total number of coverages applied to the test items by the 56-kip twin-wheel configuration.
- 22. The basic test data used in this analysis are summarized in table 2. In addition, data used in this analysis but obtained from related investigations are also shown in table 2. These additional data are recorded and discussed in reference 2. Each test was assigned a test number for easy reference. For each test conducted, the following data are shown: test number; test section number and mat type; lane and item number; load per wheel; tire inflation (gage) pressure; tire contact area (measured); tire contact pressure (obtained by dividing the load on a tire by the measured contact area); twin-wheel spacing; tire size and ply rating; coverages; type of failure (either subgrade, strengthening layer, or borderline); rated CBR for subgrade and overlying strengthening layer (a minimum of three determinations were made at each depth increment, and these values were generally averaged for all increments within the subgrade and strengthening layer to obtain a rated CBR value for each); required total flexible pavement thickness (see paragraph 20); actual thickness of strengthening layer; landing mat thickness reduction (see paragraph 19); and effective thickness (thickness reduction plus actual thickness of strengthening layer).
- 23. From a comparison of results of single- and twin-wheel tests of test section I, twin loads are supported by the mat to a coverage level beyond that anticipated and to the degree that the two loads are supported as

well as (and even somewhat better than) one wheel of the twin configuration acting alone. This can be seen by comparing results of single-wheel tests 2, 3, and 4 with results of twin-wheel tests 9, 10, and 11. Based on past experience, it might be expected that twin wheels spaced at 4.01 radii, as was the case in tests 9, 10, and 11, would act almost the same as one wheel of the twin. (In prior tests conducted at WES, 5 a 50,000-1b twin-wheel load with a center-to-center spacing of 5.4 radii was no more severe than a 25,000-1b single-wheel load). The reason for the twin wheel outperforming the single wheel is not apparent.

24. Plate 6 is a plot of required flexible pavement thickness versus effective thickness. This plot includes all single- and twin-wheel subgrade failure points and borderline failures, as presented in table 2. Borderline failures are landing mat failures, the causes of which cannot be directly attributed to either the subgrade or to the strengthening layer beneath the mat. Only subgrade and borderline failures were used in analysis because the approach to the analysis utilizes the flexible pavement CBR design concept, which is based on a total thickness requirement above a known-strength subgrade. As would be expected, the data grouped according to mat strength (stiffness), which is reflected by the value of the mat thickness reduction. A line of equality (solid line) is shown in plate 6, and this line is a good average for the data. Thus, it can be concluded that the required flexible pavement thickness is equal to the effective thickness of the mat-soil structure. However, for design purposes for thickness requirements for strengthening soils beneath landing mat, it is felt that a conservative line through the data is justified. Thus, a limiting line (dotted line) is shown in plate 6, and this line is proposed for use in the establishment of design criteria for landing-mat-surfaced airfields. The equation of this line is as follows:

$$t_{um} = (0.2875 \log C + 0.1875) \sqrt{\frac{P}{8.1 \text{ CBR}} - \frac{A}{\pi}} - TR$$
 (2)

where

t = total thickness of strengthening soil under mat, in.
C = number of coverages

P = single- or equivalent single-wheel load, 1b

CBR = measure of subgrade strength

A = tire contact area, sq in.

TR = mat thickness reduction, in.

Conclusions

25. From the analysis of test results reported herein, it was concluded that thickness criteria for strengthening soils beneath landing mat can be expressed by equation 2. The equation is based on a conservative analysis of the data presented in plate 6. This mathematical expression represents the complete pattern of basic strength requirements for landing-mat-surfaced airfields for single- and multiple-wheel loadings.

Recommendations

- 26. Based on the results of this study, the following recommendations are made:
 - a. Design and evaluation curves based on the design criteria developed herein should be developed for landing-mat-surfaced airfields. Typical curves are shown in Appendix C.
 - <u>b</u>. From a practical construction standpoint, a minimum strengthening layer thickness of 6 in. should be used wherever required.
 - c. Although not presently essential to the development of adequate design criteria, additional studies and tests should be made to determine more precisely the relationship between single- and multiple-wheel loads applied to landing-matsurfaced soils.

Literature Cited

- 1. U. S Department of Defense, "Unified Soil Classification System for Roads, Airfields, Embankments, and Foundations," MIL-STD-619B, 12 June 1968, Government Printing Office, Washington, D. C.
- 2. Thompson, A. B. and Burns, C. D., "Criteria for Designing Runways To Be Surfaced with Landing Mat and Membrane-Type Materials," Technical Report No. 3-539, Apr 1960, U. S. Army Engineer Waterways Experiment Station, CE, Vicksburg, Miss.
- 3. Ladd, D. M. and Ulery, H. H., Jr., "Aircraft Ground-Flotation Investigation; Part 1, Basic Report," Technical Documentary Report AFFDL-TDR-66-43, Aug 1967, Air Force Flight Dynamics Laboratory, Wright-Patterson

- Air Force Base, Ohio; also published as Technical Report No. 3-737, Aug 1967, U. S. Army Engineer Waterways Experiment Station, CE, Vicksburg, Miss.
- 4. Ahlvin, R. G., "Developing a Set of CBR Design Curves," Instruction Report No. 4, Nov 1959, U. S. Army Engineer Waterways Experiment Station, CE, Vicksburg, Miss.
- 5. Burns, C. D. and Fenwick, W. B., "Development of CBR Design Curves for Runways To Be Surfaced with M8A1 (Formerly T10) Steel Landing Mat," Miscellaneous Paper No. 4-817, May 1966, U. S. Army Engineer Waterways Experiment Station, CE, Vicksburg, Miss.

Table 1
Summary of Tire Data

Lane	Wheel Assembly	Assembly Load, 1b	Tire Size			Contact Pressure psi	Inflation Pressure psi							
		<u>Te</u>	st Section	I, M8A1	Mat									
1	Single	31,000	56x16	32	208.1	149	185							
2	Twin	56,000	56x16	24	257.7	109	105							
2,3	Twin	62,000	56x16	32	200.5	155	185							
Test Section II, XML8 Mat														
1	Single	30,000	30x11.5	24	128.5	234	250							
2	Twin	70,000	44x16	28	192.1	182	185							
		Tes	t Section	III, XM1	8 Mat									
1	Single	25,000	30x11.5	24	111.0	225	250							
2	Single	75,000	25.00x28	30	648.5	116	125							
		<u>Te</u>	st Section	IV, XMI	8 Mat									
1	Single	60,000	25.00x28	30	538.2	112	125							
		<u>1</u>	est Sectio	n V, AM2	Mat									
1	Single	25,000	30x11.5	24	111.0	225	250							

Table 2 Traffic Test Results

rst	lter	loui For Acol	Tire In- :lation Pressure psi	Tire Contact Area sq in.	Contact Fres- sure	_ Spac	sheel ing radii	Tire Size	Tire Fly hat- ing	Cover-	ailures Dre	Fat Sub- grade	ed CFR Strength- ening Layer	(t) in.	(t _{un})	(Tk) in.	(TH + t _{um})
								Test Co	tion I	, 4 %)	<u>unt</u>						
Lane 1	1 2 3	21 31 51	185 185 185	503 503 509	11.9 11.9 11.9		 	56x16 56x16 56x16	35 35 35	310 310	Lubgrade Borderline Strengthening layer	2.3 2.0 3.2	7 8	19.6 22.4 23.9	20 20	16.6 16.6 16.6	22.6 28.6 35.6
Lane	<u>.</u>	4	187	509	11,0		••	56r16	42	. 30		3.2	7	25.0	24	16.,	40.**
!	1	26 26 26 26	15, 10, 10, 10	නු8 නු8 නු8 නු අ	109 109 109 109	32 32 32	3.53 3.53 3.53 3.53	56x16 56x10 56x14 56x16	24 24 24	120 612 1091 1324	Subgride Borderline Strengthening layer	2.3 2.d 2.9 2.8	550 07	36.2 36.3 37.6 35.6	6 12 20 24	20.7 20.7 20.7 20.7	29,-7 32-7 40.7 44.7
10	2	31 31	185 185	200 200	155 155	5 25	4.01 4.01	56x16 50x16	32 32	120 468	Cubgrade Strengthening layer	2.3 3.0	6 7	32.7 33.7	12	17.1 17.1	29.1 37.1
13	•	31	185	200	155	32	4.01	56x16	32	750	14/01	3.0	7	37.6	54	17.1	41.1
								Tert Sec	tion I	1, XM18	Mat						
12 13 15 15 Lane	2	30 30 30	250 250 250 256	191 131 131 131	55.) 55.) 55.0 55.0	 		30x11.5 30x11.5 30x11.5 30x11.5	2'. 24 24	72 170 202 202	Subgrade Subgrade Borderline Strengthening layer	1.3 2.3 1	3.0 3.1 3.4 3.4	30.6 26.3 34.7 22.4	7 12 17 35	22.4 22.4 22.4 22.4	29.1. 34.1. 39.1. 57.1
16 27 13 49	2 .	36 76, 76 35	185 185 185 18	193 193 193 193	180 180 180 182	32.5 .2.5 .32.5	4.21 4.21 4.21	Luxió Luxió Luxió Luxió Luxió	28 26 28 28	32 60 141 300	Subgrade Subgrade Borderlin: Strengthening layer	1.4 1.7 1.7	3.3 5.1 3.4 3.7	35.8 35.8 44.2 29.5	7 12 17 35	29.5 29.5 29.5 29.5	39.5 41.6 46.9 e)9
								Test Sect	13n II	1, 20028	Unt						
Lane 20 21 Inne	1	න න	ළ ඉ ආ 0	111 111	22) 22)			30x11.5 30x11.5	24 24	528 834	Subgrade Subgrade	2.1 2.0	? 7	29.4 32.1	6 12	21.6 21.6	27.6 33.0
2? 2; 2'.	1 2	75 75 75	125 125 125	.49 .49	116 116 11	::	 	25.00x3 25.00x3 25.00x3	30 30 30	5f 72 92	Subgrade Subgrade Lubgrade	2.1 1.8 1.9	8 7 6	35.3 40.6 41.1	., 12 17	30.0 37.0 21.0	10.0 40.0 51.0
								Test Jes	tion 1								
2)		,0	125	138	112	••		21,00128	30	348 V. AX2	Subgrade	1.8	7	1	15.5	10.0	• • • • •
2%		۵,	~ ,0	111	25			30/11.	2,1	230	Subgrade	3.7	,	20.6	•	18.5	24.5
									<u> 177 Xa</u>		•						
2% 2% 29 20 21		క్రభు	100 200 300 300 300	232 145 112 112 112	104 172 223 222 222	 		56x16 56x16 56x16 56x16 56x16	ટ. જ. જ.	302 86 70 960 150	Cubgrade Cubgrade Cubgrade Cubgrade Cubgrade	5.0 5.7 20	37 31 86 56 83	17.6 14.2 11.8 8.5 12.9	6** 6 3	17.5 10.2 7.2 7.2 7.2	23.5 16.2 18.2 10.2 13.2
									es un	<u>t</u>							
53 35 35 37 47 43		88855555 8885555 88855	200 200 300 200 200 300 300 300	252 164 112 179 270 187 187	107 172 223 106 185 277 257			96x16 96x16 96x16 96x16 96x16 96x14 96x16	સંસ્થાન સાથે સાથે સાથે સાથે સાથે સાથે	316 227 78 40 300 40 170 400	Subjrade Subgrade Cubgrade Subgrade Subgrade Subgrade Subgrade Subgrade	5.2 4.8 5.9 2.6 5.3 4.3 15	29 57 19 33 19 49 92 72	16.9 17.9 12.9 20.5 22.8 19.2 12.4 13.5	12 12 3	18.5 11.2 d.2 21.5 12.6 d.4 8.4	25 17.2 12 27.5 27.6 20.4 11.4 16.4
			• • •						190 X1	_							
40 41 42 43 44	::	8 8 8 8 8 8 8 8	100 200 300 300 300	112 113 114 5-5	108 172 223 223 223			54x16 56x16 56x16 56x14 54x16	તે. સંસ્થા સ	160 59 359 400 600	Subgrade Subgrade Subgrade Subgrade Jubgrade	3.4 9.5 15 17	27 30 100 41, 100	16.9 16.4 15.5 9.8 9.5	6 12 3 6	17.5 10.2 7.2 7.2 7.2	24.5 16.2 19.2 10.0 13.2

Note: (t) - 'equired flexible payment thickness; (t_{un}) = actual thickness of strengthening layer; (78) = mat thickness reduction; (70 + t_{un}) = effective thickness.

* Into for test runders 27 through 44 were obtained from tables 2 and 3 of reference 2 (see literature Cited at end of main text).

** Strengthening layer consisted at a high-quality, graded, crushed limestone.

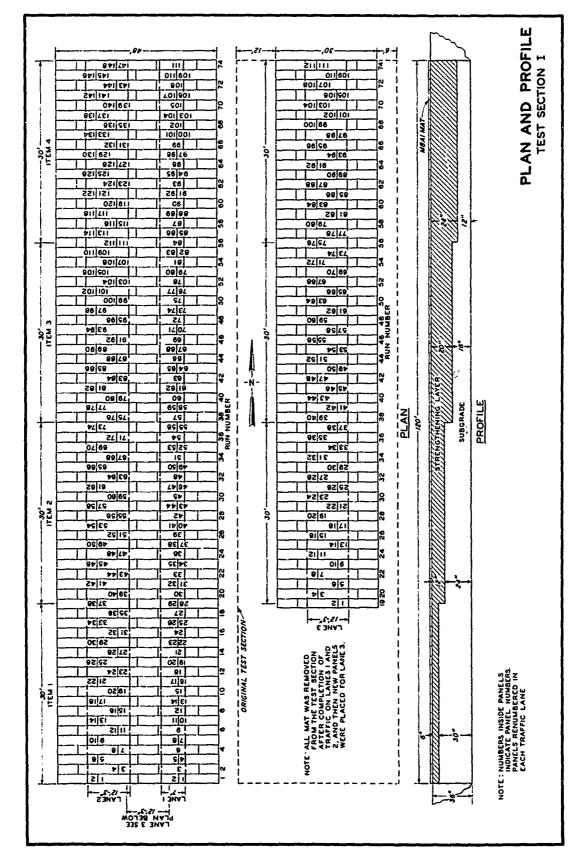


PLATE I

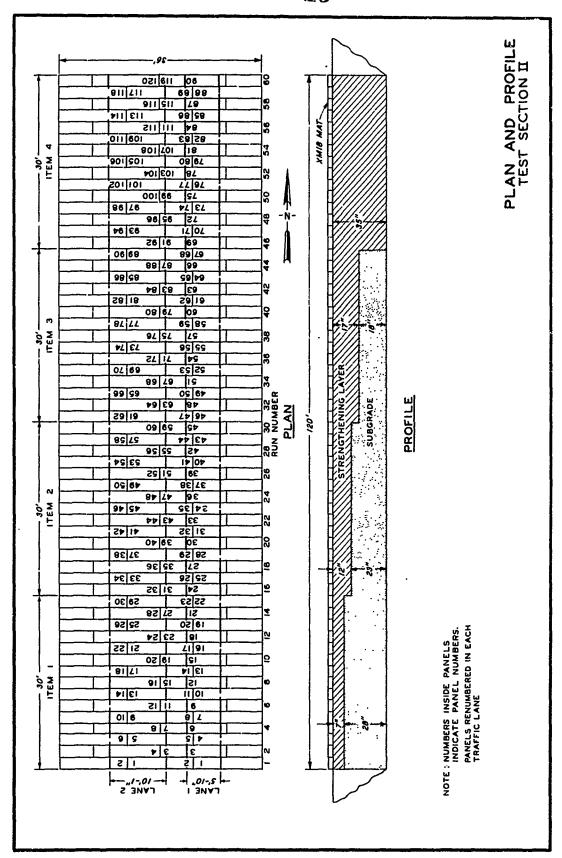
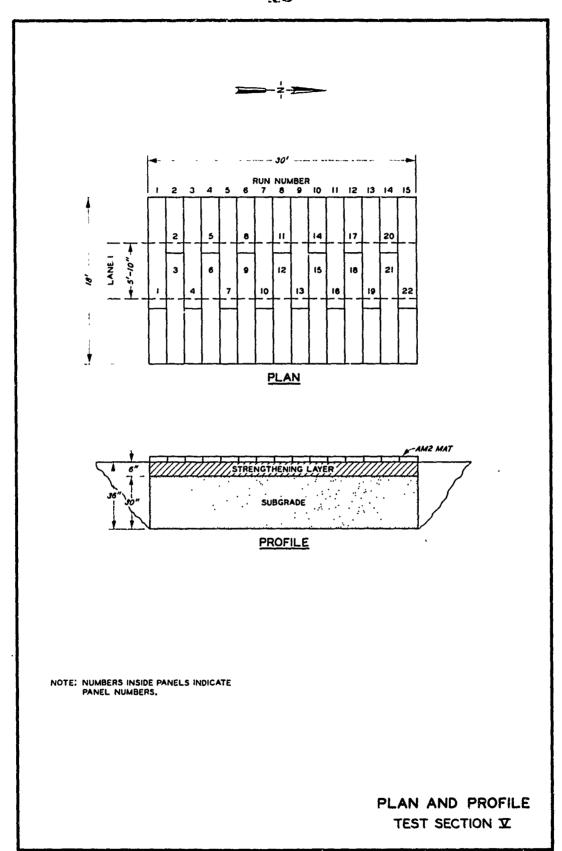


PLATE 2

* ** ***

PLATE 3



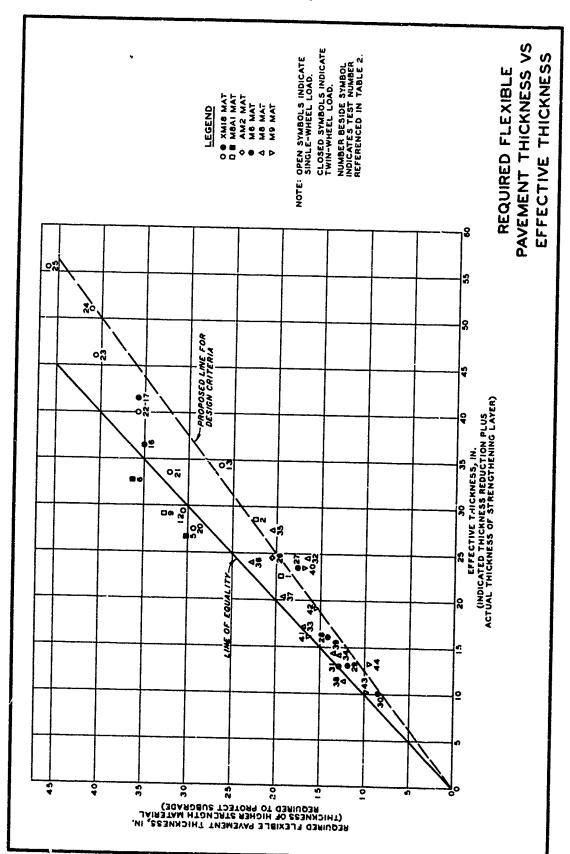


PLATE 6

Appendix A: Traffic Tests

1. Five test sections were constructed and trafficked under controlled conditions to provide the performance data required to develop a method for determining soil thickness requirements for landing-mat-surfaced airfields. Details of the traffic tests are documented in this appendix.

Test Section I

2. A plan and profile of test section I are shown in plate Al. The plan of lane 3 is shown separately in plate Al for clarity. Traffic distribution patterns for lanes 1, 2, and 3 are shown in plate A2. A 31,000-lb single-wheel load utilizing a 56:16 tire with an inflation pressure of 185 psi was used to traffic lane 1. Lane 2 was trafficked with a 56,000-lb twin-wheel load utilizing 56xl6 tires with an inflation pressure of 105 psi. After 612 coverages, the load was increased to 62,000 lb with an inflation pressure of 185 psi. Lane 3 was trafficked with a 62,000-lb twin-wheel load utilizing 56xl6 tires with an inflation pressure of 185 psi. Soils and mat breakage data for the test section are summarized in tables Al and A2, respectively.

Lane 1

- 3. Item 1. A view of item 1 prior to traffic is shown in photograph Al. There was considerable permanent deformation of the mat with the application of traffic, and by 10 coverages, longitudinal deformation averaged about 1.9 in. Traffic was continued to 30 coverages, at which time item 1 was considered failed due to excessive roughness. At failure, three panels had cover-plate weld breaks. Photograph A2 shows permanent deformation of 3.2 in. at one location; average deformation was about 2.3 in. for the test item at 30 coverages. An overall view of item 1 at failure is shown in photograph A3.
- 4. Item 2. An overall view of item 2 prior to traffic is shown in photograph A4. Permanent deformation of the mat was observed immediately after application of traffic, and after 30 coverages, longitudinal deformation averaged 1.1 in. Mat breakage was first observed at approximately

70 coverages with the occurrence of locking-lug breaks. At 110 coverages, six locking lugs had broken, and permanent deformation and roughness had become excessive. Item 2 was considered failed at 110 coverages, and an overall view of the item at failure is shown in photograph A5.

- 5. Item 3. An overall view of item 3 prior to traffic is shown in photograph A6. Deformation of the mat, which did not deform so rapidly as that in items 1 and 2, averaged about 0.8 in. after 30 coverages. Deformation increased very slowly with increased traffic. Mat breakage was first observed at approximately 170 coverages with the occurrence of two locking-lug breaks. As traffic continued, mat breakage and deformation increased rapidly, and at 310 coverages, item 3 was considered failed. Deformation at failure averaged about 2 in. Failure was due primarily to mat breakage. Ar overall view of item 3 at failure is shown in photograph A7.
- 6. Item 4. An overall view of item 4 prior to traffic is shown in photograph A8. Permanent mat deformation developed slowly during the application of traffic. The first mat breakage, locking-lug breaks, was observed at approximately 230 coverages. Mat breakage progressed with increased traffic, and after 430 coverages, item 4 was considered failed due to excessive mat breakage. Photograph A9 shows a typical locking-lug break, and an overall view of item 4 at failure is shown in photograph A10.
- 7. Permanent deformation. Permanent deformation of each item in lane 1, as determined from level readings taken prior to and at intervals throughout the traffic test period, is shown in plate A3. The deformation values in plate A3 are averages of two readings taken in each item. Centerline profiles, illustrating deformation of the mat along the center line of each test item, are shown in plate A4.
- 8. <u>Mat deflection</u>. Deflections of the mat surface under static load were determined from level readings and are shown in plate A5. Deflections are shown for two panel locations prior to traffic and at failure of each test item.

Lane 2

9. Item 1. An overall view of item 1 prior to traffic is shown in photograph All. Permanent mat deformation developed at a uniform rate with the application of traffic. After 120 coverages, the subgrade had deformed

sufficiently to consider item 1 failed. An overall view of item 1 at failure is shown in photograph Al2.

- 10. Item 2. An overall view of item 2 prior to traffic is shown in photograph Al3. Permanent deformation of the mat developed very slowly with the application of traffic. By 120 coverages, longitudinal deformation averaged about 1.0 in. Mat breakage was first observed at 360 coverages. Breakage developed slowly, and after 612 coverages, only three breaks were observed. At this time, item 2 was considered failed because of excessive roughness. An overall view of item 2 at failure is shown in photograph Al4.
- 11. Item 3. An overall view of item 3 prior to traffic is shown in photograph Al5. No apparent distress was observed in item 3 during 612 coverages. After 612 coverages, the test load was increased from 56,000 to 62,000 lb, and the tire inflation pressure was increased from 105 to 185 psi. Photograph Al6 shows item 3 at 612 coverages prior to trafficking with the revised test load. The first sign of distress, a locking-lug break, was noted after an additional 192 coverages had been applied. After 312 additional coverages (924 coverages of mixed traffic), tire hazards had developed from excessive mat breakage, as shown in photograph Al7, and item 3 was considered failed. An overall view of item 3 at failure is shown in photograph Al8. Item 3 received a total of 924 coverages.
- 12. Item 4. An overall view of item 4 prior to traffic is shown in photograph Al9. The mat showed no signs of distress by 612 coverages, and additional traffic was applied with the revised load described in the preceding paragraph. After an additional 288 coverages, the first mat break was observed. After 350 additional coverages, mat breakage began to develop rapidly, and after 432 additional coverages (1044 total coverages), item 4 had developed serious tire hazards and was considered failed. An overall view of item 4 at failure is shown in photograph A20. Item 4 received a total of 1044 coverages.
- 13. <u>Permanent deformation</u>. Permanent deformation of each item in lane 2, as determined from level readings taken prior to and at intervals throughout the traffic test period, is shown in plate A6. The deformation values are averages of two readings taken in each item. Center-line

profiles, illustrating deformation along the center line of each test item, are shown in plate A7.

14. <u>Mat deflection</u>. Deflections of the mat under static load were determined from level readings and are shown in plate A8. The deflections shown are for two panel locations prior to traffic and at failur of each test item.

Lane 3

- 15. After traffic had been completed on lanes 1 and 2, the M8Al landing mat was removed from the test section, and new panels of M8Al were placed so that lane 3 was positioned in the untrafficked area between lanes 1 and 2 as shown in plate Al. Item 1 was not trafficked in lane 3.
- 16. Item 2. An overall view of item 2 prior to traffic is shown in photograph A21. As traffic was applied, permanent deformation of the mat was fairly rapid, and after 120 coverages, item 2 was considered failed due to excessive roughness. There was no mat breakage at failure. An overall view of item 2 at failure is shown in photograph A22.
- 17. Item 3. An overall view of item 3 prior to traffic is shown in photograph A23. Permanent deformation developed slowly with continued traffic. The first mat breakage was observed at 228 coverages. Traffic was continued to 408 coverages, at which time item 3 was considered failed due to excessive roughness. An overall view of item 3 at failure is shown in photograph A24.
- 18. Item 4. An overall view of item 4 prior to traffic is shown in photograph A25. Mat deformation developed slowly, and the first mat breakage was observed at approximately 385 coverages. With increased traffic, roughness began to develop, and at 750 coverages, item 4 was failed due to excessive roughness and tire hazards caused by mat breakage. An overall view of item 4 at failure is shown in photograph A26.
- 19. <u>Permanent deformation</u>. Permanent deformation of each item in lane 3, as determined from level readings taken prior to and at intervals throughout the traffic test period, is shown in plate A9. The deformation values in plate A9 are averages of two readings taken in each item. Centerline profiles, illustrating deformation of the mat along the center line of each test item, are shown in plate A10.

20. <u>Mat deflection</u>. Deflections of the mat surface under static load were determined from level readings and are shown in plate All. The deflections are shown for two locations prior to traffic and at failure of each test item.

Test Section II

21. A plan and profile of test section II are shown in plate Al2. Traffic distribution patterns and tire characteristics for lares 1 and 2 are shown in plate Al3. A 30,000-lb single-wheel load utilizing a 30x11.5 tire with an inflation pressure of 250 psi was used to traffic lane 1. Lane 2 was trafficked with a 70,000-lb twin-wheel load utilizing 44x16 tires with an inflation pressure of 185 psi. Soils and mat breakage data are summarized in tables Al and A3, respectively. Lane 1

- 22. Item 1. An overall view of item 1 prior to traffic is shown in photograph A27. Mat roughness in item 1 was observed after three coverages, and after 12 coverages, dishing measured 0.75 in. in the transverse direction in one location, as shown in photograph A28. Photograph A29 shows longitudinal deformation of about 1 in., as measured on panel 13. By 32 coverages, the bottom lip of the overlapping end joint in panel 5 had sheared off, causing panel 4 to deflect under the wheel load. After 54 coverages, the C-rail of panel 4 was extensively damaged (see photograph A30), and panels 4 and 5 were replaced. After 72 coverages, item 1 was considered failed due to excessive roughness. An overall view of item 1 at failure is shown in photograph A31.
- 23. Item 2. An overall view of item 2 prior to traffic is shown in photograph A32. Permanent mat deformation was observed at approximately eight coverages. By 100 coverages, longitudinal deformation averaged about 1.7 in. After 170 coverages, item 2 was considered failed due to excessive roughness. Longitudinal deformation at failure averaged 1.8 in. No mat breakage was observed in item 2 throughout the period of traffic. An overall view of item 2 at failure is shown in photograph A33.
 - 24. Item 3. An overall view of item 3 prior to traffic is shown in

photograph A34. Permanent mat deformation, observed initially after several coverages, increased slowly with continued traffic. By 100 coverages, longitudinal deformation averaged 1.0 in. The first sign of mat breakage, an internal rib failure, was observed at 142 coverages. After 202 coverages, item 3 was considered failed due to excessive roughness caused by subgrade deformation. An overall view of item 3 at failure is shown in photograph A35.

- 25. Item 4. An overall view of item 4 prior to traffic is shown in photograph A36. Permanent mat deformation developed slowly as traffic was applied. Longitudinal deformation averaged 1.3 in. after 100 coverages and increased to 2.5 in. by 202 coverages. After 202 coverages, item 4 was considered failed due to excessive deformation of the mat. At failure one panel was observed with internal rib failures, and one panel was observed with a damaged C-rail. An overall view of item 4 at failure is shown in photograp! A37.
- 26. <u>Permanent deformation</u>. Permanent deformation of the mat for each test item in lane 1, as determined from level readings taken prior to and at intervals throughout the traffic test period, is shown in plate Al4. Level readings were recorded from two locations in each test item, and the curves in plate Al4 were plotted from average deformation measurements. Center-line profiles illustrating deformation of the mat along the center line of each test item are shown in plate Al5.
- 27. <u>Mat deflection</u>. Deflections of the mat surface under static load were determined from level readings and are shown in plate Al6. The deflections are shown for two panel locations prior to traffic and at failure of each test item.

Lane 2

28. Item 1. An overall view of item 1 prior to traffic is shown in photograph A38. Permanent deformation was observed after the first coverage and developed very rapidly with increased traffic. By 32 coverages, longitudinal deformation averaged 2.3 in., and item 1 was considered failed. At failure, three panels had disconnected along the C-rail and male connectors, as shown in photograph A39. The bottom lip of an overlapping end joint had also sheared in one panel. An overall view of item 1

at failure is shown in photograph A40.

- 29. Item 2. An overall view of item 2 prior to traffic is shown in photograph A41. Longitudinal deformation measured 1.13 in. after 32 coverages. Mat breakage was first observed after 48 coverages. The bottom lip of an overlapping end joint had sheared on one panel, and two panels had disconnected along the C-rail and male connectors. Item 2 was considered failed after 60 coverages due to excessive deformation and mat breakage. Photograph A42 shows a typical overlapping end-joint failure. An overall view of item 2 at failure is shown in photograph A43.
- 30. Item 3. An overall view of item 3 prior to traffic is shown in photograph A44. No serious mat damage occurred in item 3 during the first 60 coverages; however, longitudinal deformation averaged 1.2 in. The first mat failure occurred at 120 coverages, at which time two panels had disconnected along the C-rail and male connectors. By 144 coverages, excessive mat breakage, as shown in photograph A45, had occurred and longitudinal deformation had increased to 3.1 in., as shown in photograph A46. Failure of item 3 at this coverage level was attributed to excessive mat deformation and mat breakage caused by subgrade deformation. An overall view of item 3 at failure is shown in photograph A47.
- 31. Item 4. An overall view of item 4 prior to traffic is shown in photograph A48. Little deformation and no mat breakage were observed in item 4 prior to 144 coverages. At 228 coverages, the bottom lip of an overlapping end connector sheared. Permanent deformation started to develop very rapidly after 228 coverages, and after 300 coverages, item 4 was considered failed due to excessive longitudinal deformation (2.8 in., see photograph A49) and mat breakage. An overall view of item 4 at failure is shown in photograph A50.
- 32. <u>Permanent deformation</u>. Permanent deformation of the mat for each test item in lane 2, as determined from level readings taken prior to and at intervals throughout the traffic test period, is shown in plate Al7. The plots in plate Al7 show the differential deformation of the mat at failure. Center-line profiles, illustrating deformation along the center line of each test item, are shown in plate Al8.
 - 33. Mat deflection. Elastic deflections of the mat surface under

static load were determined from level readings and are shown in plate Al9. The deflections are shown for two panel locations prior to traffic and at failure of each test item.

Test Section III

- 34. A plan and profile of test section III are shown in plate A20. Traffic distribution patterns and tire characteristics for lanes 1 and 2 are shown in plate A21. A 25,000-lb single-wheel load utilizing a 30x11.5 tire with an inflation pressure of 250 psi was used to traffic lane 1. Lane 2 was trafficked with a 75,000-lb single-wheel load utilizing a 25.00x28 tire with an inflation pressure of 125 psi. Soils and mat breakage data are summarized in tables A1 and A3, respectively. Lane 1
- 35. Item 1. A view of item 1 prior to traffic is shown in photograph A51. The first sign of mat breakage, noted at 42 coverages, was a weld crack along the underlapping end joint of panel 29. At 200 coverages, the weld crack had progressed along the wiith of the panel; however, the panel was not considered a tire hazard at this time (see photograph A52). At this coverage level, similar weld cracks had also developed in panels 28 (adjacent to panel 29) and 14. As traffic continued, top skin tears developed parallel to the C-rail in panels 28 and 29, and at 314 coverages, the panels were considered failed and were replaced. A surface depression, indicative of an internal rib failure, had also developed in panel 30; however, the damage was slight and did not warrant removal of the panel. Portions of panels 28, 29, and 30 are shown in photograph A53.
- 36. Traffic was resumed after replacement of failed panels 28 and 29, and at 374 coverages, five additional panels showed evidence of internal rib failures. The rib failures developed slowly with continued traffic, but no serious damage occurred until 500 coverages had been completed. At this point, the mat began to deteriorate very rapidly. The top lip of the underlapping end joint of panel 14 sheared off at 526 coverages, and at 528 coverages, the locking bar between panels 13 and 14 was forced from the panels, as shown in photograph A54. Item 1 was considered failed at

528 coverages due to excessive roughness and mat breakage. An overall view of item 1 at failure is shown in photograph A55.

- 37. Item 2. A view of item 2 prior to traffic is shown in photograph A56. No distress was noted in item 2 until surface depressions, indications of internal rib failures, developed in panels 51 and 54 at 400 coverages. At 477 coverages, a top skin tear developed at a location corresponding to an internal rib failure in panel 51. Photograph A57 shows the skin tear at 528 coverages. Panels 51 and 54 were considered failed at 569 coverages and were replaced. Several additional panels had developed evidence of internal rib failures at this time; however, the damage was slight, and the panels were not considered hazardous to tires during aircraft operations. At 720 coverages, top skin tears and depressions from internal rib failures in panel 48 became a serious tire hazard, as shown in photograph A58. The top lip of the underlapping end joint in panel 47 had also sheared off at this time, and both panels were replaced. Traffic was continued to 884 coverages before the entire test item was considered failed. Photographs A59 and A60, respectively, show depressions from internal rib failures and top skin tears of typical failed panels. Failure of item 2 was attributed to excessive mat breakage (rib failures and skin tears). An overall view of item 2 at failure is shown in photograph A61.
- 38. <u>Permanent deformation</u>. Permanent deformation of the mat, as determined from level readings taken prior to and at the end of traffic, is shown in plate A22. Since the mat was laid in a staggered pattern, an end joint in every other run of mat was located on the center line of the traffic lane. In adjacent runs, the center of a panel was located on the center line of the traffic lane. Plate A22 shows the average cross section for both conditions for each test item of the test lane. These data indicate that the deformation across the traffic lane was generally about the same regardless of where the joint was located. Subgrade deformation at failure is shown in plate A23. Center-line profiles, illustrating deformation of the mat along the center line of each test item, are shown in plate A24.

THE THE STATE OF T

1

39. <u>Mat deflection</u>. Deflections of the mat surface under static load were determined from level readings and are shown in plate A25. The

deflections shown are for two panel locations prior to traffic and at failure of each test item.

Lane 2

40. Item 1. An overall view of item 1 prior to traffic is shown in photograph A62. Evidence of internal rib failures developed in 18 panels at approximately 11 coverages. At 16 coverages, the top lip of the overlapping end joint sheared off in panel 17. The resulting failure, as shown in photograph A63, was a tire hazard and the panel was replaced. At 48 coverages, a similar failure occurred in panel 29. Panel 32 was also failed at 48 coverages due to internal rib failures. The panels were replaced, and traffic was continued to 56 coverages, at which time the entire test item was considered failed. Failure of the test item was attributed chiefly to the failure of the end joints, although approximately 50 percent of all of the panels had internal rib failures. Typical mat failure is shown in photograph A64. An overall view of item 1 at failure is shown in photograph A65.

41. Item 2. A view of item 2 prior to traffic is shown in photograph A66. Depressions caused by internal rib failures were first noted at 16 coverages. The damage was slight, however, and did not present a hazard to continued operation. At 28 coverages, the bottom lip of the overlapping end joint of panel 41 sheared off. Five additional panels were damaged in a similar manner after 60 coverages. These failures in turn led to C-rail failures of the panels in the adjacent runs. Traffic was stopped at 72 coverages, and item 2 was considered failed due to excessive roughness and mat breakage. An overall view of item 2 at failure is shown in photograph A67.

42. Item 3. An overall view of item 3 prior to traffic is shown in photograph A68. Breakage was first noted at approximately 16 coverages, at which time depressions that indicated internal rib failures developed. The damaged ribs, however, did not present a tire hazard throughout the period of traffic. The first major distress occurred at 22 coverages when the bottom lip of the overlapping end joint sheared on panels 101 and 113. A similar break occurred in panel 97 at 48 coverages, as shown in photograph A69. In photograph A69, it can be seen that the panel adjacent to panel 97 is

depressed. This depression resulted from support loss caused by the failed overlapping end joint. Traffic was continued to 92 coverages, and although no panels were replaced during the traffic test period, the entire test item was failed at 92 coverages due to roughness and tire hazards caused by end-joint failures. An overall view of item 3 at failure is shown in photograph A70.

43. Permanent deformation. Permanent deformation of the mat, determined from level readings taken prior to and at the end of traffic, is plotted in plate A26 as the differential deformation at failure. The average cross section for each test item is shown in plate A26. Cross sections of subgrade deformation for items 1, 2, and 3 are shown in plate A27. Center-line profiles, illustrating deformation along the center line of each test item, are shown in plate A28.

44. <u>Mat deflection</u>. Deflections of the mat surface under static load were determined from level readings and are shown in plate A29. The deflections are shown for two locations.

Test Section IV

45. A plan and profile of test section IV are shown in plate A30. The test section consisted of only one test item and was trafficked with a 60,000-lb single-wheel load utilizing a 25.00x28 tire with an inflation pressure of 125 psi. The traffic distribution pattern and tire characteristics for the test section are shown in plate A31. Soils and mat breakage data are summarized in tables A1 and A3, respectively.

46. An overall view of the test section prior to traffic is shown in photograph A71. A depression caused by the failure of an internal rib was the first sign of mat breakage. After 42 coverages, damaged ribs were apparent in 18 panels. The damage was slight, however, and traffic was continued. After approximately 190 coverages, it was observed that the bottom lip of the overlapping end joint had completely sheared off panel 33. A similar break was noted in panel 1 after 216 coverages. After 232 coverages, panel 33 was considered a tire hazard for further aircraft operation and was replaced. Panel 35, in the run adjacent to the run containing

panel 33, also had extensive C-rail damage and was replaced after 232 coverages. After 298 coverages, two additional panels were failed and replaced due to the failure of the bottom lip of the overlapping end joint and damaged C-rails. Traffic was continued until completion of 348 coverages, at which time the test section was considered failed due to excessive mat breakage. Photographs A72 and A73 show typical views of top skin tears at failure. An overall view of the test section at failure is shown in photograph A74.

- 47. Permanent deformation of the mat was determined from level readings taken prior to and at the end of traffic. Plate A32 shows the differential deformation at failure. A cross section of the subgrade deformation at failure is shown in plate A33. A center-line profile, illustrating deformation along the center line of the test section, is shown in plate A34.
- 46. Deflections of the mat surface under static load were determined from level readings and are shown in plate A35. The deflections are shown for two locations prior to traffic and at failure of the test section.

Test Section V

- 49. A plan and profile of test section V are shown in plate A36. The test section consisted of one test item and was surfaced with one-, two-, and three-piece AM2 landing mat and was trafficked with a 25,000-lb single-wheel load utilizing a 30xll.5 tire with an inflation pressure of 250 psi. The traffic distribution pattern and tire characteristics for the test section are shown in plate A37. Soils and mat breakage data are presented in tables Al and A3, respectively.
- 50. An overall view of the test section prior to traffic is shown in photograph A75. Mat breakage was first observed at approximately 75 coverages, with end-joint weld breaks occurring in three panels. By 100 coverages, the breaks averaged about 5 in. in length. After 120 coverages, two additional panels developed similar weld breaks, and after 140 coverages, the end joint of one panel had completely sheared off. The panel was replaced, and traffic was continued. A similar failure occurred after 210 coverages. By 330 coverages, a total of 10 panels had failed due to weld

breaks. In four of these panels, the end connector had completely sheared off. A typical weld break is shown in photograph A76. Traffic was stopped at 330 coverages, and the test section was considered failed due to excessive mat breakage. Photograph A77 shows an overall view of test section V at failure.

- 51. Permanent deformation of the mat was determined from level readings taken prior to and at the end of traffic. Cross-section plots showing the differential deformation at failure are shown in plate A38. A center-line profile, showing the differential deformation at failure along the center line of the test section, is shown in plate A39.
- 52. Deflections of the mat surface under static load were determined from level readings and are shown in plate A40. The deflections are shown for two locations prior to traffic and at failure of the test section.

これのないからないないないないないとのできないと

Table Al
Supmary of CBR, Water Content, and Dry Density Data

are	Test Itez	Traffic Cover-	Depth	<u>CBA</u>	Water Content	Dry Density pef	Test Lane Item		Pepth in.	CBE	Water Content	Dry Density pef	Lane	Test Item	Traffic Cover-	Depth in.	CER	Water Content	Dry Densit, pcf
		Tes	t Sect	ion I				Test Se	etien I	(Con	inued)				Test	Section	n III		
1	1	٥	0 8 12	5 1.5 2.9	27.9 32.9 32.5	91.3 84.1 86.3	(Cont'd)	408	0 8 22 28	8 8 3.4 2.9	25.4 25.2 28.9 30.9	95.0 96.0 87.4 88.0	1	1	0	6 12 18	8 1.5 1.9 2.8	26.7 34.3 34.7 34.8	93.2 83.7 83.6 83.1
	2	30	0 8 14 0	6 2.0 2.6 5	27.5 33.1 31.8 26.9	93.6 84.3 86.4 92.4	L,	0	0 6 12 18	6 6 7 7	27.7 26.6 25.5 26.7	95.5 93.7 94.1 92.2			528	0 6 12 18	6 1.8 1.9 2.7	26.8 34.4 34.4 34.4	94.0 84.0 84.2 83.9
		110	6 14 20 0	9 2.5 3.5 6	26.4 32.5 32.0 25.7	93.5 86.7 88.0 93.7		750	26 30 0	2.2 3.1 5	32.5 29.6 25.4	86.1 88.8 94.1		2	٥	0 6 12	8 4.0 1.5	26.0 28.4 35.2	93.0 90.7 82.~
	•		6 14 20	6 2.7 2.6	26.7 31.6 31.5	94.8 87.5 87.4			8 16 26 30	8 6 2.5 4.0	27.8 27.2 31.7 30.0	95.6 92.8 87.8 88.5			528	94 94 18	1.8 1.8	35.5 35.5 25.5 27.8	A1.7 82.2 94.7 91.5
	3	٥	0 8 14 22 28	8 9 3.3 2.3	27.2 26.3 25.6 28.8 32.8	93.9 96.7 95.4 89.2 87.8) 1	<u>Test</u> O	Section 0 8	2.9 1.1	32.3 38.7	84.5 78.8			334	12 18 14 0	2.5 2.1 1.9	34.4 36.4 36.3 25.8	83 0 82.4 82.4 94.9
		310	0 8 14 22	7 8 8 4.7	25.1 25.4 25.1 25.8	93.8 95.7 94.9 91.3		72	16 24 9	1.0 1.8 3.2 1.3	39.8 38.7 32.3 35.1	76.5 78.1 86.2 80.2				54 19 55	7 2.1 2.5 1.7	28.2 33.6 34.5 35.5	83.3 85.3 84.5 82.6
	14	0	28 0 6	2.3 5 7	31.4 27.5 26.6	87.3 95.3 92.5	2	o	16 24 0	0.9 1.7 2.3	37.6 38.9 33.5	78.4 78.5 84.4	2	1	0	0 12 16	8 1.5 1.9 2.8	25.7 34.3 34.1 34.5	93.2 83.7 83.6 63.1
			18 26 30	6 6 2.4 3.1	25.7 26.3 31.6 28.5	94.6 92.7 86.6 89.5			6 12 18 24	3.2 2.2 2.3 2.0	32.6 34.8 35.5 37.6	85.4 80.2 81.5 80.7			56	0 6 .2 18	7 2.3 2.2 1.8	25.5 33.8 33.8 34.6	91.1 84.6 83.9
1	4	430	6 12 18	8 6	24.5 25.6 24.3 26.8	96.5 95.4 94.7 93.2		170	0 6 14 22	3.0 3.9 2.8 2.2	34.1 32.2 35.3 37.0	86.5 86.6 80.6 82.2	2	2	e	0 6 12	8 4.0 1.5	26.0 28.4 35.2	93.0 90.7 82.2
2	1	26 0	25 30 0 8	3-1 4-1 5 1-9	31.2 29.5 27.7 33.6	88.9 91.8 93.4 83.6	3	0	0 8 16 24	2.8 3.5 3.2 1.4	33.5 32.1 32.9 38.8	24.8 86.9 86.9 78.6			72	18 24 0 6	1.6 1.8 1.0 5.4	35.9 35.5 25.1 27.9	81. 82. 96.
		120	14 0 8 14	3.0 4.9 2.1 2.2	32.5 27.4 30.9 32.5	86.4 92.9 83.9 86.0		202	20 6 12 0	3.5 2.9 4.2 1.1	31.6 32.8 31.0 39.9	86.8 87.1 85.8 77.3		3	0	12 18 24 0	2.1 1.6 2.0 8	35.7 36.6 33.1 26.4	82.6 81.1 84.7 92.6
	2	0	0 6 0 6	5 6 2.5 3.9	26.9 27.7 33.0 30.8	92.3 95.1 84.3 87.6		o	26 0 6 12	3.3 3.3 3.5	37.3 .7 .7	84.5 87.6 82.2		-		11 17 23	5 6 1.9 2.1	29.0 27.2 31.5 33.3	69. 93. 87. 84.5
		615	0 6 14	7 5 2.1	25.9 26.8 32.4	95.9 93.5 85.9	ĺ		18 0 6	4.1 3.6 2.9	31.8 32.5 32.5	85.4 84.9 85.8			92	0 11 17	8 4.6 4.1 1.8	29.4 29.4 29.0 35.8	72.6 89.5 91.6 82.2
	3	c	20 3 8 14	2.7 4.7 5	31.8 28.3 26.7 25.9	93.0 94.7 91.6	2 1	0	1:	3.4 2.5 1.3 0.8	32.4 32.6 35.1 40.5	85.2 83.4 78.8 76.7				23 it Sect	1.9 ion I\	34.5	83.1
		924	22 28 0 8	2.7 3.2 6 9	29.4 31.0 27.7 25.6	88.3 88.7 93.9 95.4		32	24 0 8 16	1.6 4.1 2.0 1.0	35.9 31.2 34.1 38.9	79.7 94.8 80.0 78.5			0	0 5 11 17	6 1,9	25.4 29.0 27.2 31.5	92.0 89.7 93.2 67.0
	L		58 55 17	7 3.2 2.4 6	27.2 30.6 32.6	92.1 87.6 87.2	2	0	6 0 6	1.7 2.1 2.8	37.6 33.8 33.1	80.1 83.3 84.2			348	23 0 5 11	2.1 6 6	33-3 26.6 29.1 28.4	93.1 89.1 89.1
	4	0	0 12 18 26	6 8 8 2.0	27.8 26.6 25.3 27.0 33.3	95.7 93.9 93.6 91.7 85.7		60	18 04	1.6 2.3 2.1 3.8	39.0 36.7 38.2 30.8	79.2 79.2 79.9 97.7			<u> 74</u>	17 23	2.7	36.1	82.9 83.
2	ı	1044	10 0 6 14	3.0 7 6 8	30.6 26.4 27.1 25.6	88.1 94.9 93.9 94.3	3	0	0 14 22	3.3 1.4 1.2 2.2	31.1 35.6 38.3 32.2	89.6 80.9 79.1 84.7			0	0 6 12 18	4.2 2.0 3.4 3.4	25.9 28.6 26.3 25.7	9% - 1 90 - 1 93 - 1 90 - 2
3	2	0	26 30	2.9 3.4 5	30.6 29.5 26.9	89.2 91.7 92.4		11%	8 16 24 0	3.6 2.0 3.6	32.0 33.7 37.4 31.3	84.8 80.9 79.2 87.4				15 0 0	3.0	24.8 27.1 25.1	97. 93. 95.
		120	14 20 0	7 2.5 3.7 8	27.0 32.7 31.4 25.1	94.3 85.5 87.8 96.1			50 15	3.3 6.0 1.4 1.3	30.5 29.1 40.2 35.1	87.2 90.0 77.1 84.3				19	5.0	25.4	93.
	3	0	6 14 20 0	1.9 1.9	27.9 32.1 31.8 27.7	94.3 86.3 86.7 93.5		0	0 6 12 18	2.3 3.4 3.8	33.9 31.1 31.8 31.5	85.6 86.6 86.3 85.3							
٠	•	-	14 22 28	7 9 3.0 2.7	25.8 25.1 11.9	95.2 93.5 88.7 88.2			6 6 12	4.2 4.1	29.8 30.5 30.9	87.6 88.2							

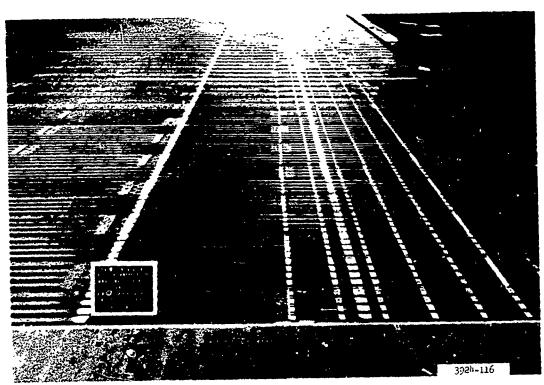
Table A2
Surmary of Traffic Test Results
Test Section I

						Mat Br	rakage					
		Strength-	CER		locking	Rolled Edge Break	Bayonet Shear	Cover- Flate Weld Break	Deflec	num Mat tion, in.		
une.	Test Item	ening Layer	Aubgrade	Traffic Coverages	Ing Break				On Joint	Center of Panel	Rating of Item at Failure	
:	1		2.3	o 30	0	0	0	o 3	1.60 2.55	1.80 2.60	Subgrade failure	
	5	I	2	0 110	2	0 1	0	0	1.58	1.50 1.65		
	3	8	3.2	310	0 18	o 6	0	6	1.20 1.45	1.15		
	4	7	2.2	0 430	6	0	0	0	0.85 1.15	1.00		
2	1	5	5•3	0 120	0	0	9	0	1.55 2.60	1.80 3.20	Sutgrade failure	
	5	6	2.8	·12	0 2	0 0	0	0 1	1.30	1.10	Subgrade failure	
	3	•	5.9	0 (12 924	0 0 12	0 0	0 0 0	0	1.20	1.09 1.09 1.58		
	4	7	2.8	107. 0	0 0 7	C 0 0	0 0 2	0 0 0	0.90 0.95 1.65	0.80 0.80 1.45		
3	2	6.	2.5	0 120	0	0	0 G	0	1.60 2.1	1.90 2.00	Subgrade failure	
	•	7	3.0	408	0 7	0	0 0	S O	1.20 1.82	1.40		
	L	7	3.0	0 150	°,	0	0	0	2.20	1.85		

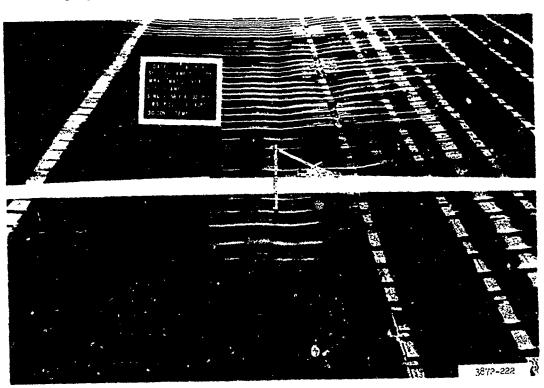
Table A3

Dummary of Traffi Teth Results

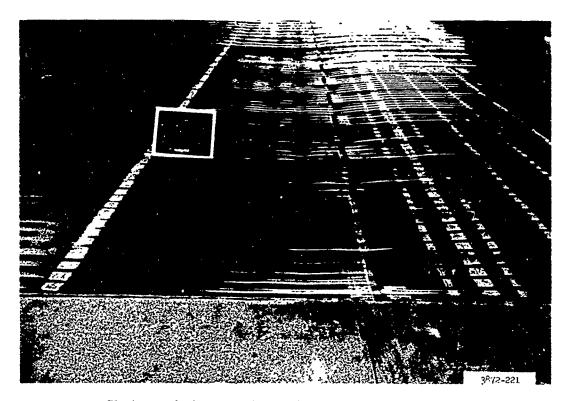
				Mat Preakast									<u></u>		
<u>Lane</u>	Test Item	Etrength- ening Layer	Sub- grade	Traffic Cover- ages	Internal tib Failures	Bottom Lip Cheared from Cverlapping End Joint	C-Rail Failure	Joint Ke. d	Top Pin	Sleared End Joint	on Joint	Print	Center of Pacel	Rating of Item at Failure	
						<u>Tes</u>	t Section	<u>II</u>							
1	1	3.0	1.3	0 72	0	0 1	9 1	0	0	0	1.55 1.69	1.62 1.28		Subgrade failure	
	2	3.1	2.3	170	0	0	0	0	0	0	1.35	0.98		Subgrade failure	
	3	3.4	1.4	505 0	0 1	0	0	0	0	0	1.15 1.20	0.90 1.25		•	
	•	3.4		505 0	0 1	0	0 1	0	0	0	1.15	0.78 0.90			
5	1	3-3	1.4	0 32	0	0 1	0	0	0	0	3.20 3.85		3.25 3.45	Subgrade failure	
	2	3.1	1.7	60	0	0 2	0	0	0	5 0	2.15 3.20		2.40 3.70	Subgrade failure	
	3	3.4	1.7	241	0	0 1	c o	0	0	0	2.00 2.85		2.35 2.90		
	i.	3.7	••	3 0 0	0	0 1	0	0	0	0	1.90 2.55		1.60 2.40		
						<u>Tes</u>	t Section	111							
1	1	7	2.1	0 528	9	0	0	o 3	0 3	0	1.04		0.98 1.05	Subgrade failure	
	5	1	2.0	0 528 884	0 1	0 6	0 2 8	0	0	0	0.68 1.08		0.68	Subgrade failure	
5	1	8	2.1	0	0 23	0	0	0	0	0	2.05		2.50 3.18	Subgrade failure	
	5	7	1.8	12	0 22	ê	9	0	0	0	1.88 2.57		1.98 2.72	Subgrade failure	
	3	•	5.0	85 0	19	0 7	7	0	0	0	1.40 2.32		1.58 2 42	Subgrade failure	
						<u>7e</u>	st Section	IA							
		7	1.8	348	န	0 10	π o	0	0 3	0	1.23 1.68		1.28 1.63	Subgrade failure	
						<u>Te</u>	st Section	<u>v</u>							
		5	3.7	0 330	0	0	0	6	0	9	0.61 0.75		0.62 0.61	Subgrade failure	



Photograph Al. Test section I, lane 1, item 1, prior to traffic



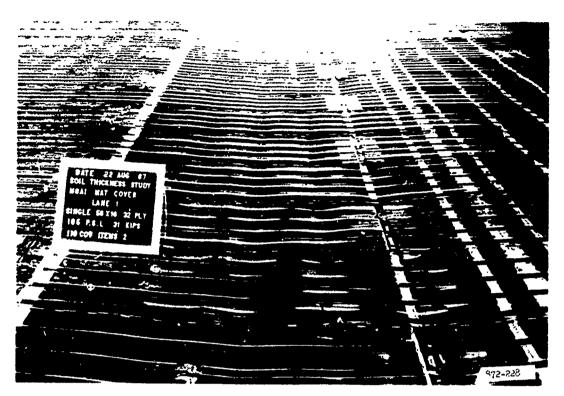
Photograph A2. Deformation of 3.2 in. in test section I, lane 1, item 1, after failure at 30 coverages



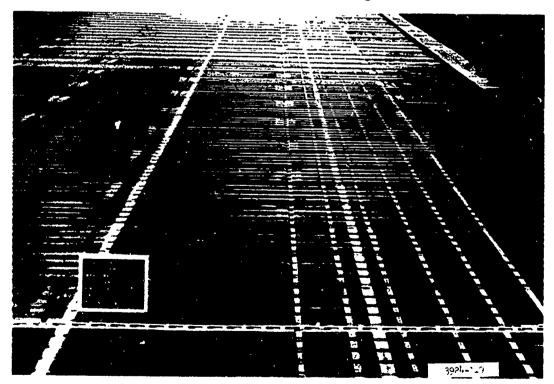
Photograph A3. Test section I, lane 1, item 1, after failure at 30 coverages



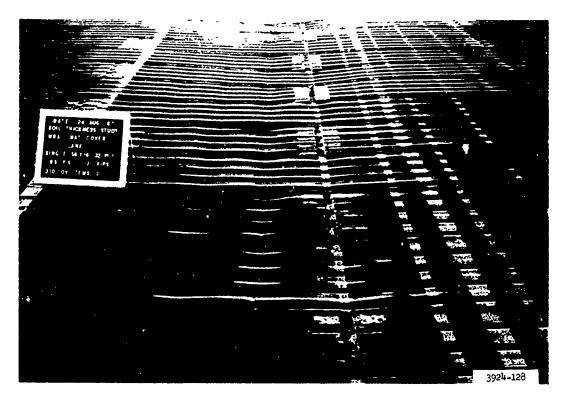
Photograph A4. Test section I, lane 1, item 2, prior to traffic



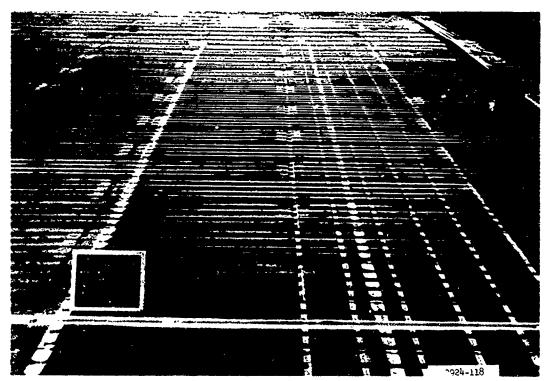
Photograph A5. Test section I, lane 1, item 2, after failure at 110 coverages



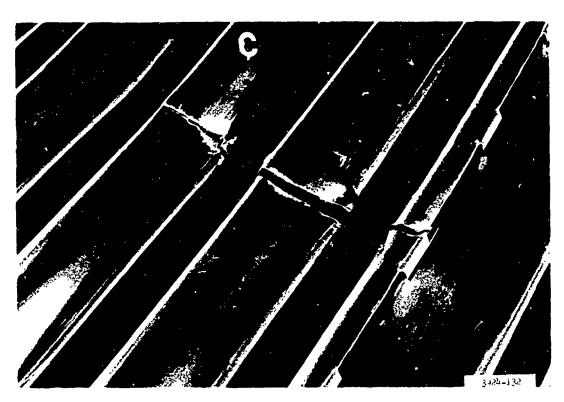
Photograph A6. Test section I, lane 1, item 3, prior to traffic



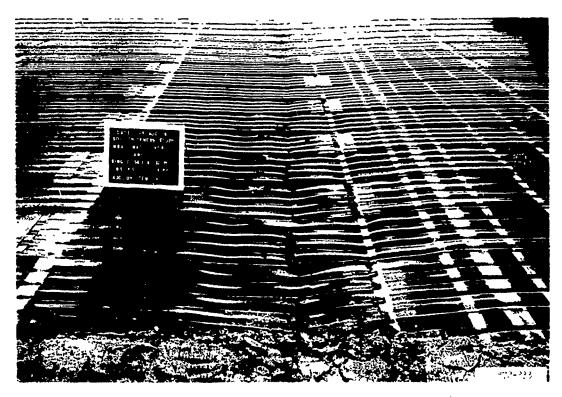
Photograph A7. Test section I, lane 1, item 3, after failure at 310 coverages



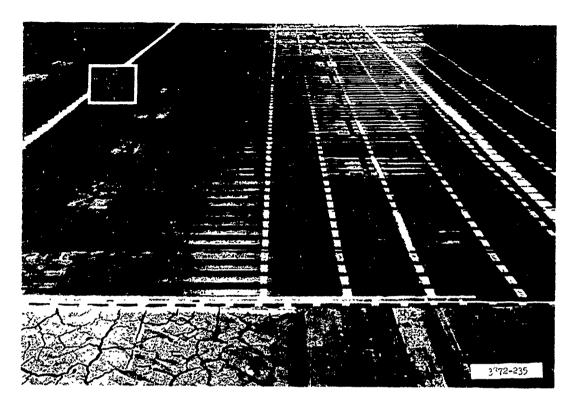
Photograph A8. Test section I, lane 1, item 4, prior to traffic



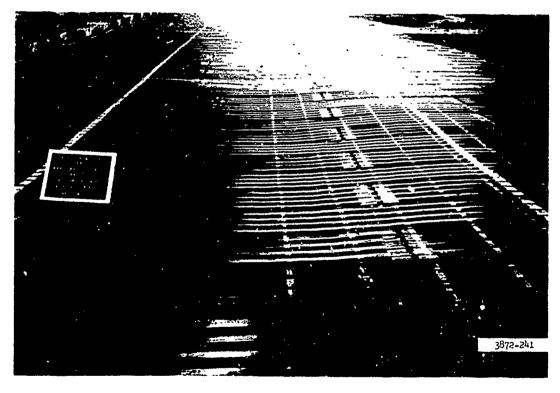
Photograph A9. Typical locking-lug break



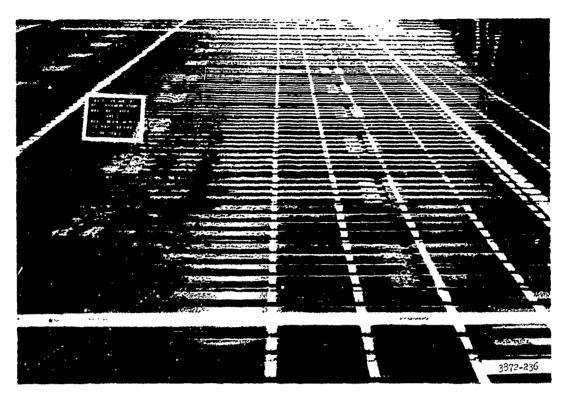
Photograph AlO. Test section I, lane 1, item 4, after failure at 430 coverages



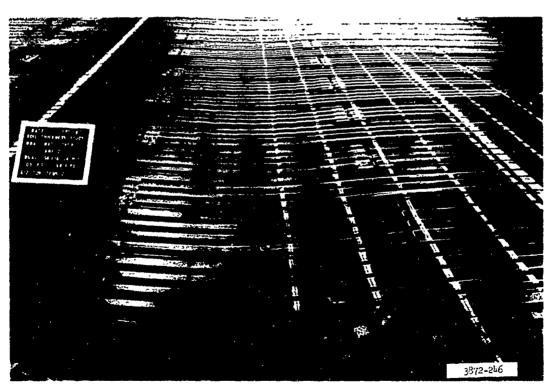
Photograph All. Test section I, lane 2, item 1, prior to traffic



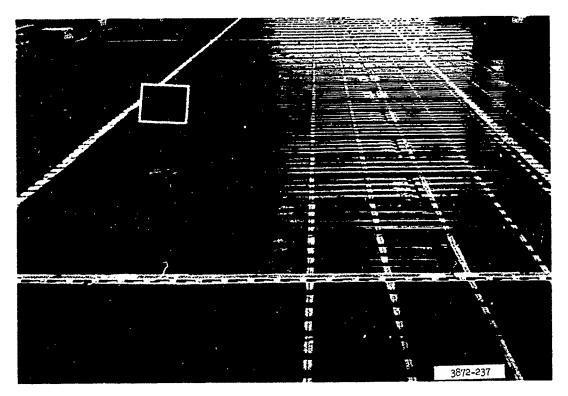
Photograph Al2. Test section 1, lane 2, item 1, after failure at 120 coverages



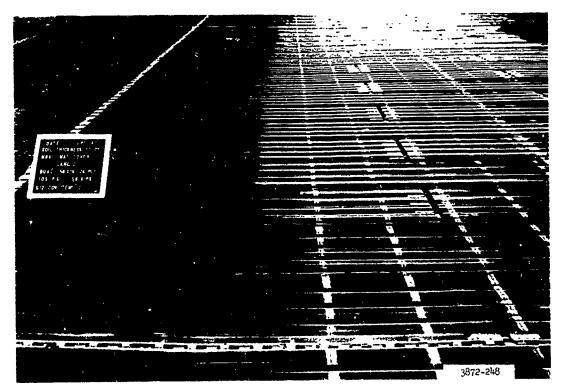
Photograph Al3. Test section I, lane 2, item 2, prior to traffic



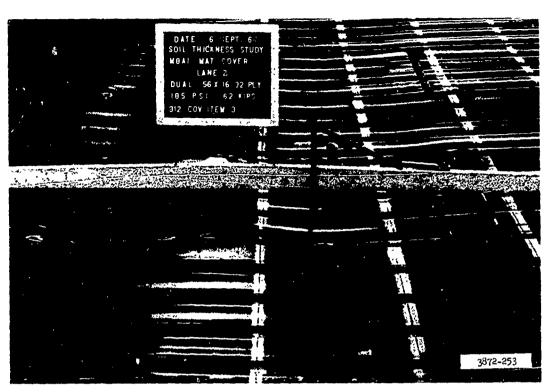
Photograph Al4. Test section I, lane 2, item 2, after failure at 612 coverages



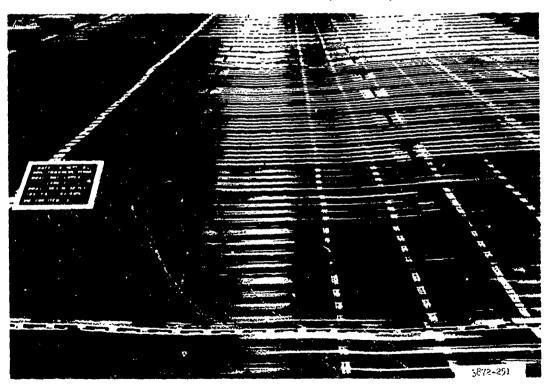
Photograph Al5. Test section I, lane 2, item 3, prior to traffic



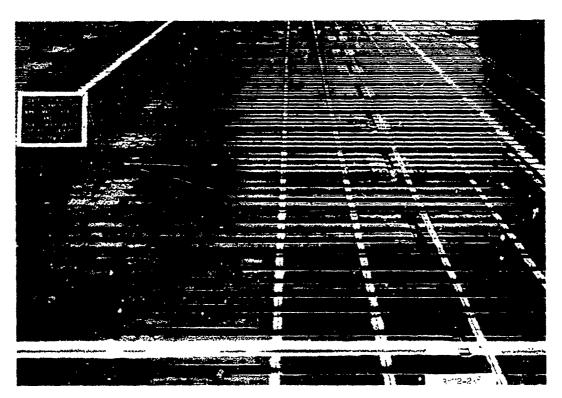
Photograph Al6. Test section I, lane 2, item 3, after 612 coverages with initial test load



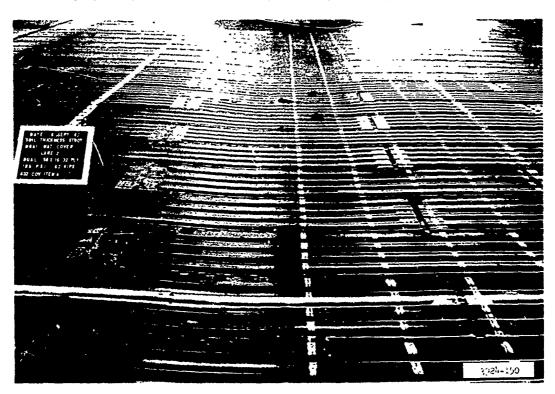
Photograph Al7. Typical mat breakage after a total of 924 coverages of mixed traffic in test section I, lane 2, item 3



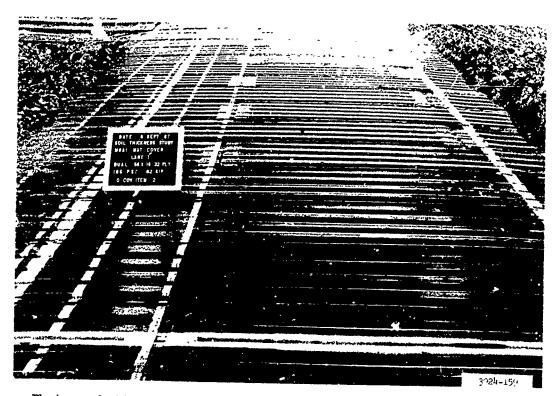
Photograph A18. Test section I, lane 2, item 3, after failure at 924 coverages of mixed traffic



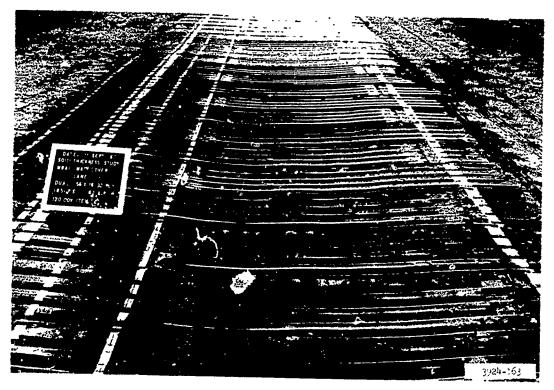
Photograph Al9. Test section I, lane 2, item 4, prior to traffic



Photograph A20. Test section I, lane 2, item 4, after failure at 1044 coverages of mixed traffic



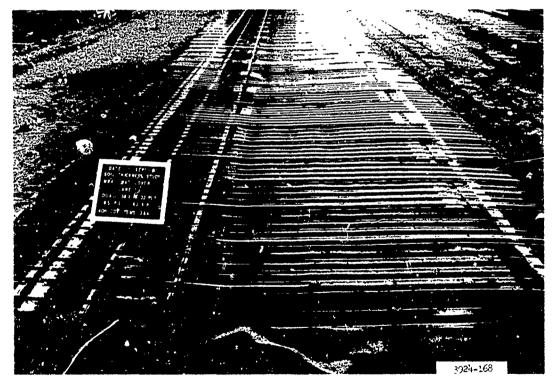
Photograph A21. Test section I, lane 3, item 2, prior to traffic



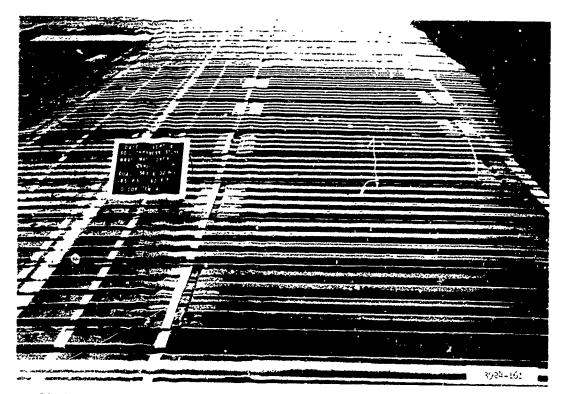
Photograph A22. Test section I, lane 3, item 2, after failure at 120 coverages



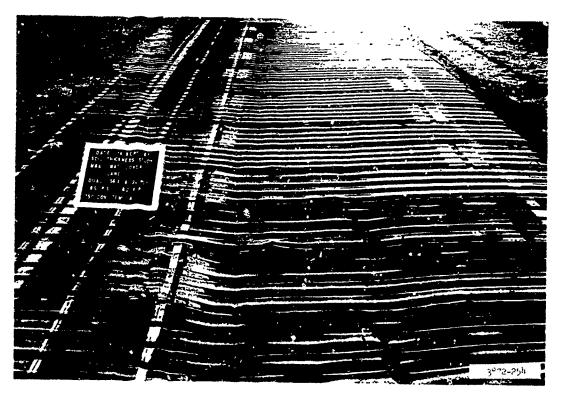
Photograph A23. Test section I, lane 3, item 3, prior to traffic



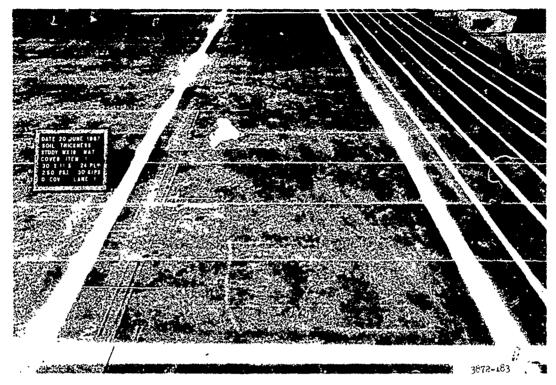
Photograph A24. Test section I, lane 3, item 3, after failure at 408 coverages



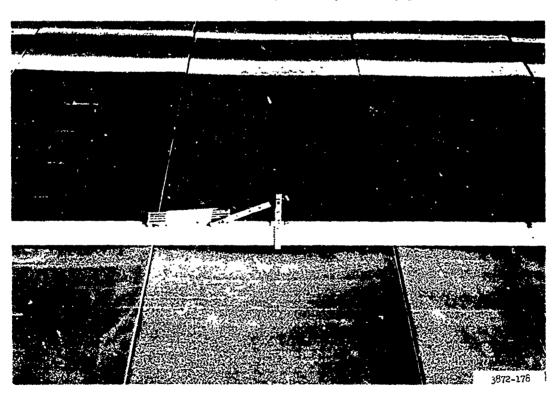
Photograph A25. Test section I, lane 3, item 4, prior to traffic



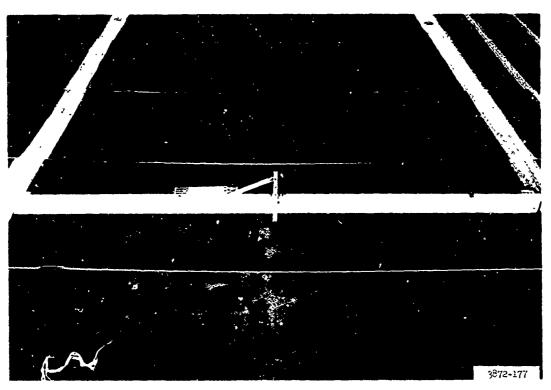
Photograph A26. Test section I, lane 3, item 4, after failure at 750 coverages



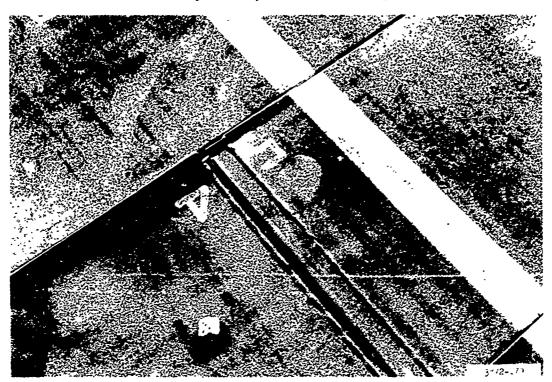
Photograph A27. Test section II, lane 1, item 1, prior to traffic



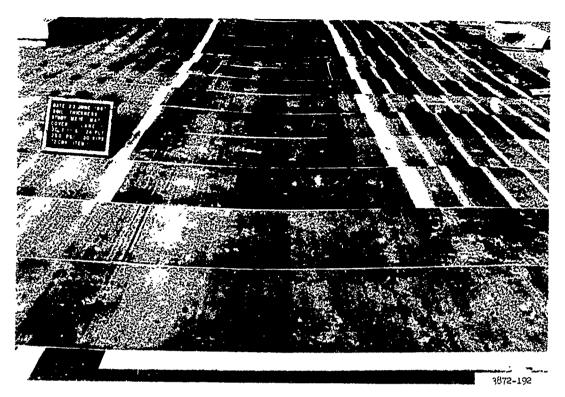
Photograph A28. Dishing of 0.75 in. in test section II, lane 1, item 1, after 12 coverages



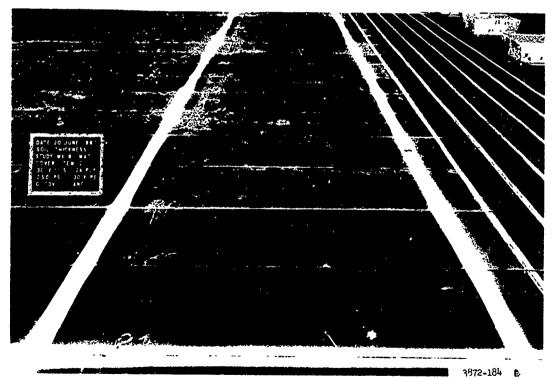
Photograph A29. Longitudinal deformation of 1 in. in test section II, lane 1, item 1, after 12 coverages



Photograph A30. Damaged C-rail of panel 4, test section II, lane 1, item 1, after 54 coverages



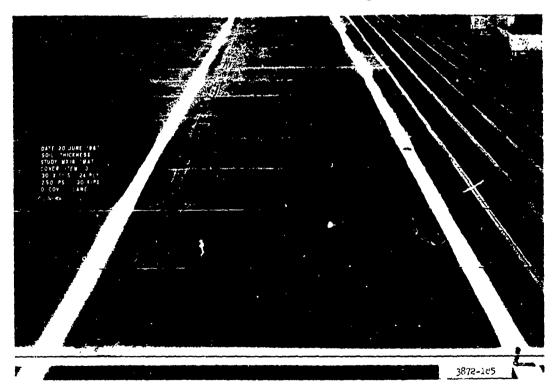
Photograph A31. Test section II, lane 1, item 1, after failure at 72 coverages



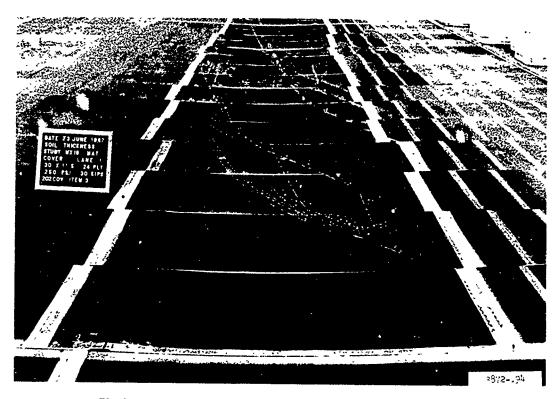
Photograph A32. Test section II, lane 1, item 2, prior to traffic



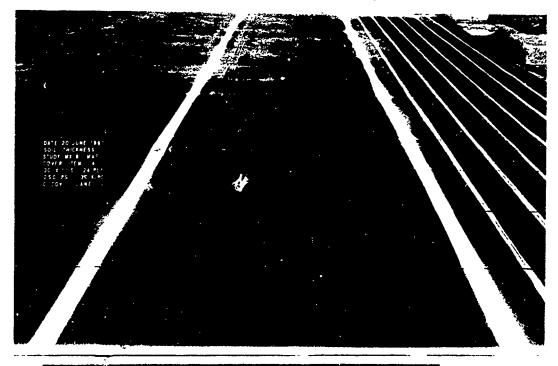
Photograph A33. Test section II, lane 1, item 2, after failure at 170 coverages



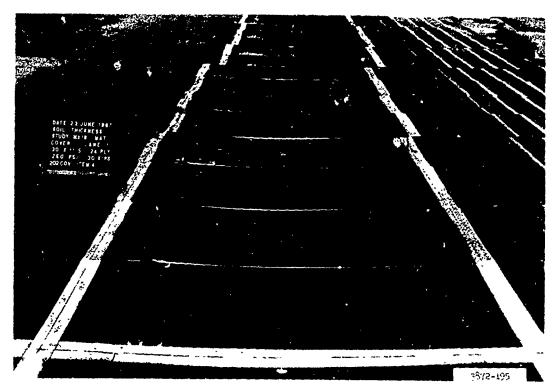
Photograph A34. Test section II, lane 1, item 3, prior to traffic



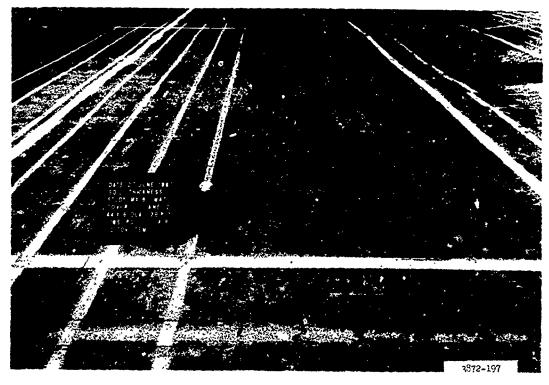
Photograph A35. Test section II, lane 1, item 3, after failure at 202 coverages



Photograph A36. Test Section B3lane 1, item 4, prior to traffic



Photograph A37. Test section II, lane 1, item 4, after failure at 202 coverages



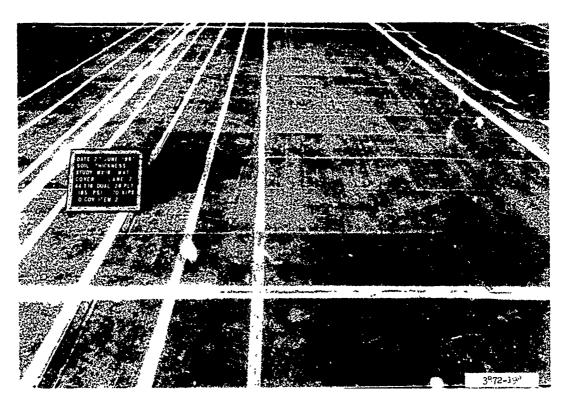
Photograph A38. Test section II, lane 2, item 1, prior to traffic



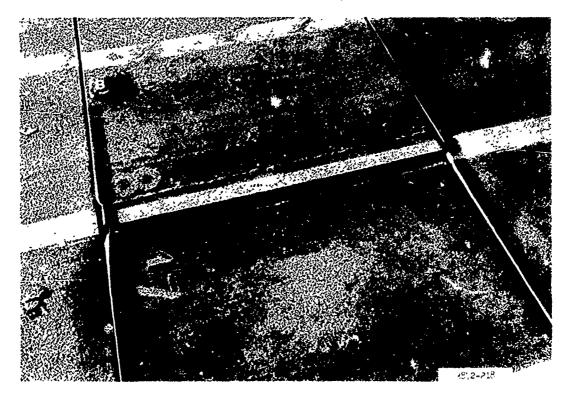
Photograph A39. Panels disconnected along C-rail and male connectors



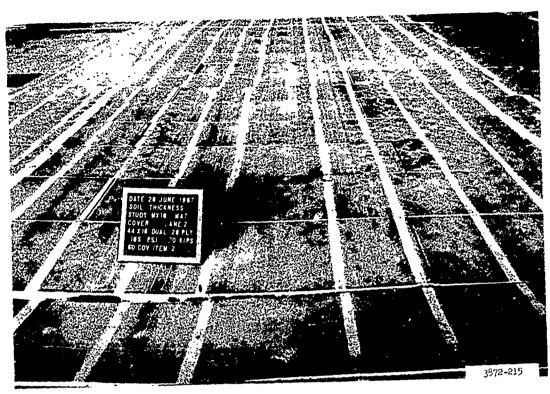
Photograph A40. Test section II, lane 2, item 1, after failure at 32 coverages



Photograph A41. Test section II, lane 2, item 2, prior to traffic



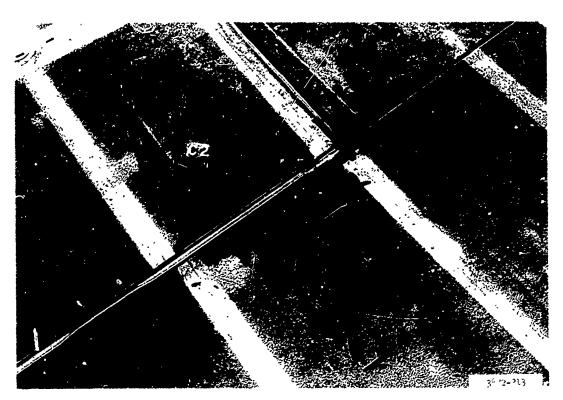
Photograph A42. Typical end-joint failure



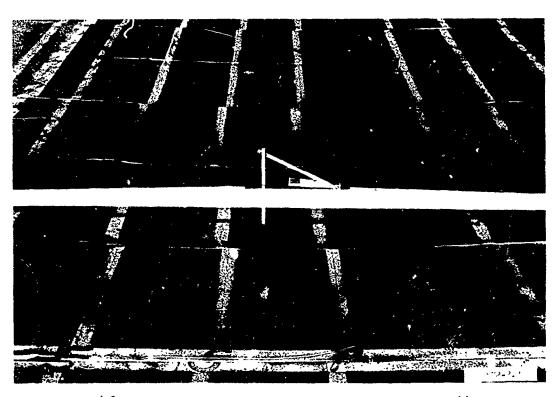
Photograph A43. Test section II, lane 2, item 2, after failure at 60 coverages



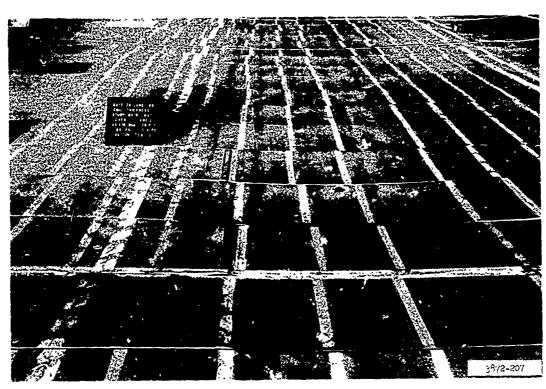
Photograph A44. Test section II, lane 2, item 3, prior to traffic



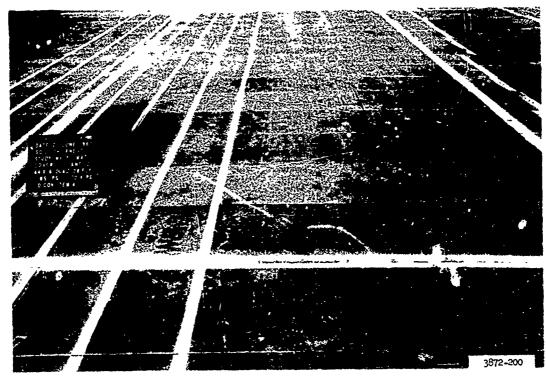
Photograph A45. Typical longitudinal and end-joint failure



Photograph A46. Longitudinal deformation of 3.1 in. after 144 coverages



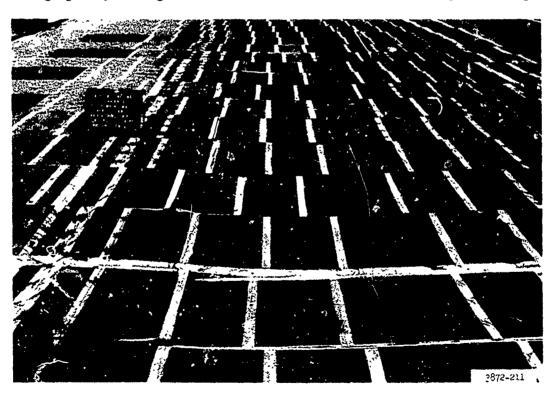
Photograph A47. Test section II, lane 2, item 3, after failure at 144 coverages



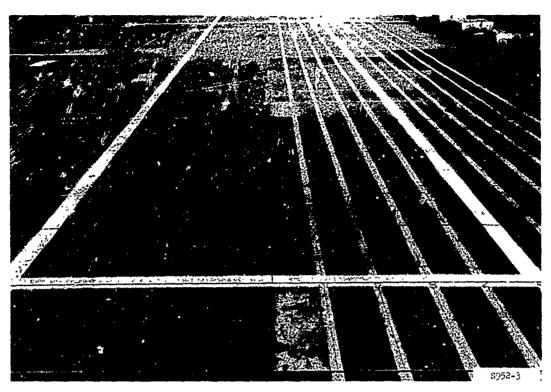
Photograph A48. Test section II, lane 2, item 4, prior to traffic



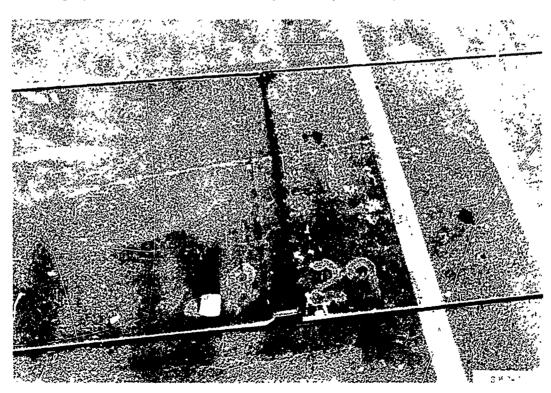
Photograph A49. Longitudinal deformation of 2.8 in. after 300 coverages



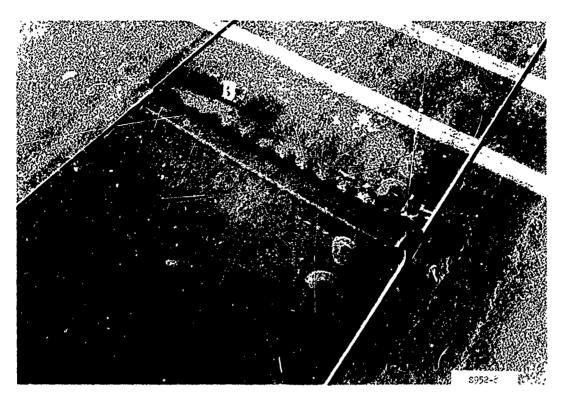
Photograph A50. Test section II, lane 2, item 4, after failure at 300 coverages



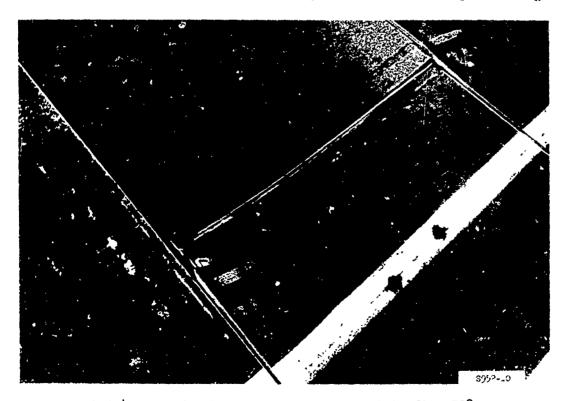
Photograph A51. Test section III, lane 1, item 1, prior to traffic



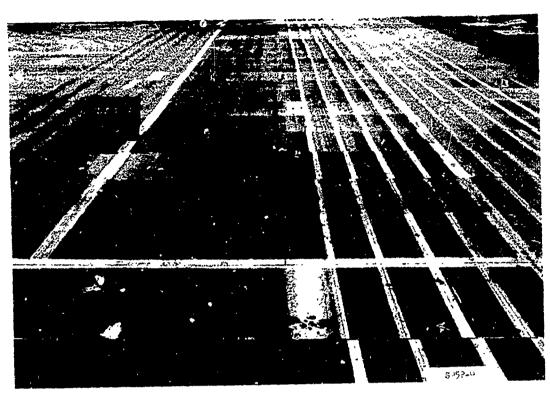
Photograph A52. Weld crack along underlapping end joint after 200 coverage:



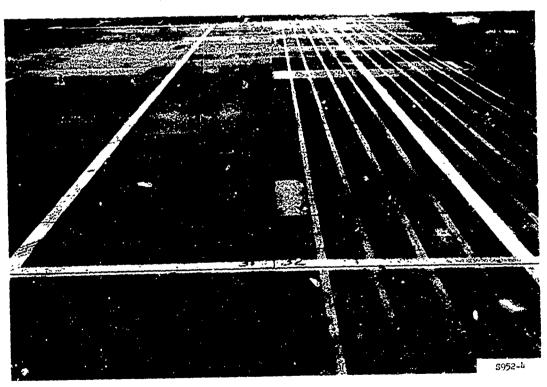
Photograph A53. Top skin tears and end-joint failure after 314 coverages



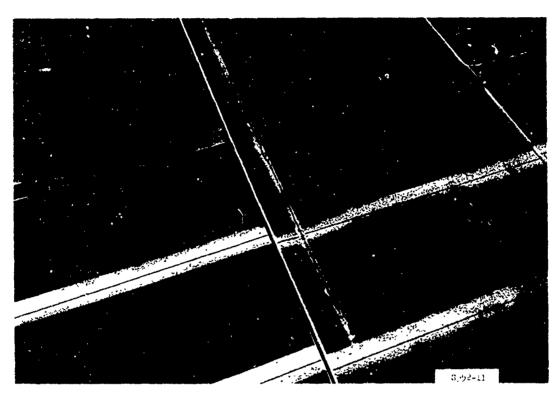
Photograph A54. Locking bar forced from end joint after 528 coverages



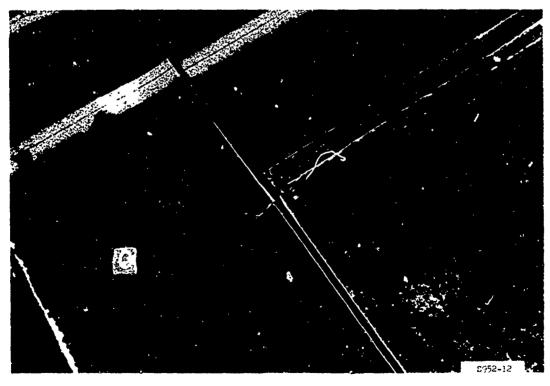
Photograph A55. Test section III, lane 1, item 1, after failure at 528 coverages



Photograph A56. Test section III, lane 1, item 2, prior to traffic



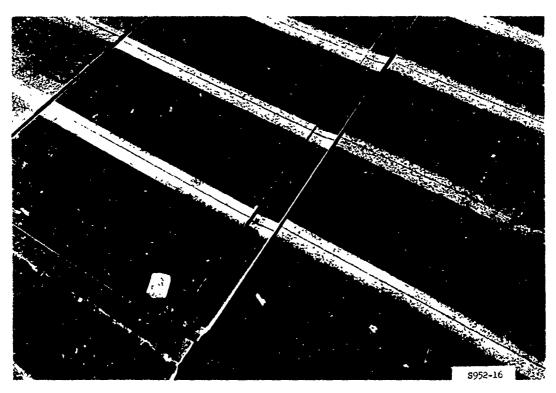
Photograph A57. Top skin tear after 528 coverages



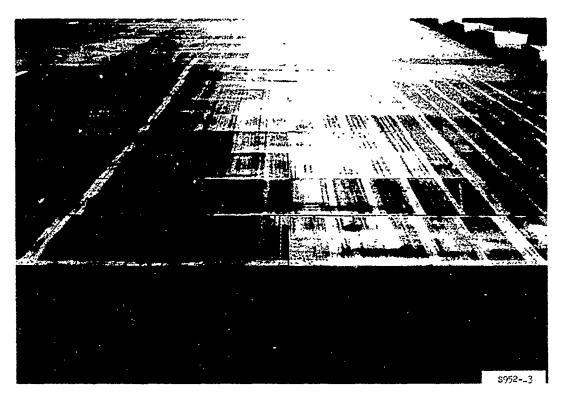
Photograph A58. Internal rib failures and top skin tear after 720 coverages



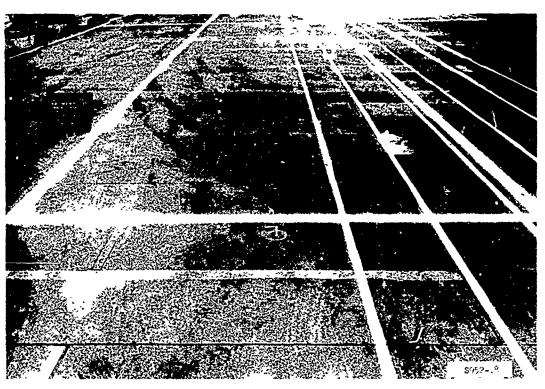
Photograph A59. Typical internal rib failures after 884 coverages



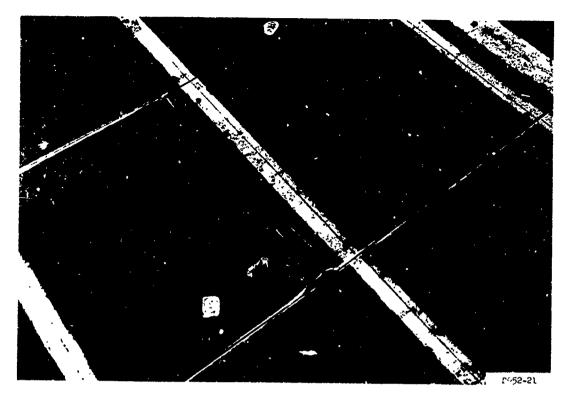
Photograph A60. Top skin tear along C-rail after 884 coverages



Photograph A61. Test section III, lane 1, item 2, after failure at 884 coverages



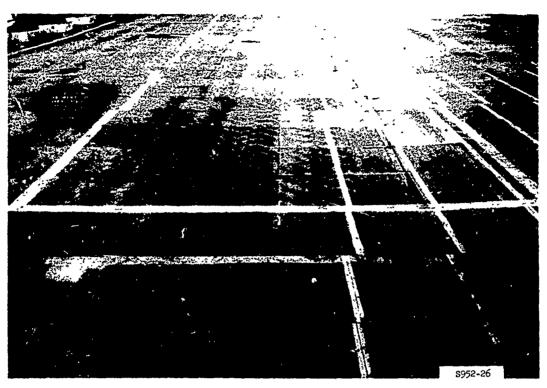
Photograph A62. Test section III, lane 2, item 1, prior to traffic



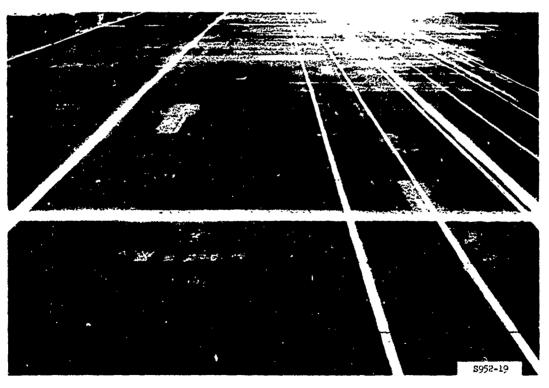
Photograph A63. Top lip of overlapping end joint sheared after 16 coverages



Photograph A64. Typical end-joint failure after 56 coverages



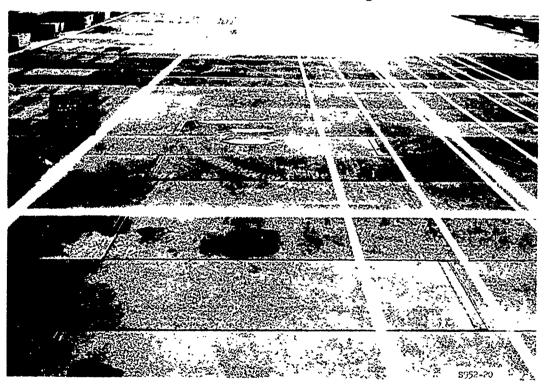
Photograph A65. Test section III, lane 2, item 1, after failure at 56 coverages



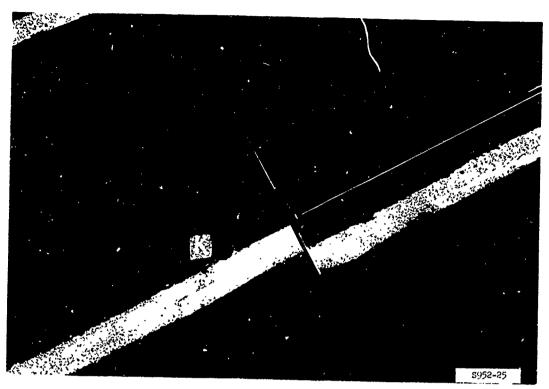
Photograph A66. Test section III, lane 2, item 2, prior to traffic



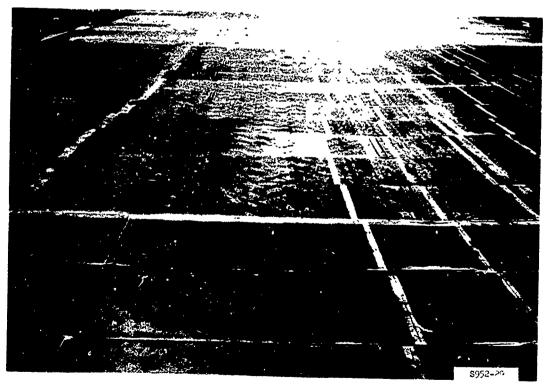
Photograph A67. Test section III, lane 2, item 2, after failure at 72 coverages



Photograph A68. Test section III, lane 2, item 3, prior to traffic



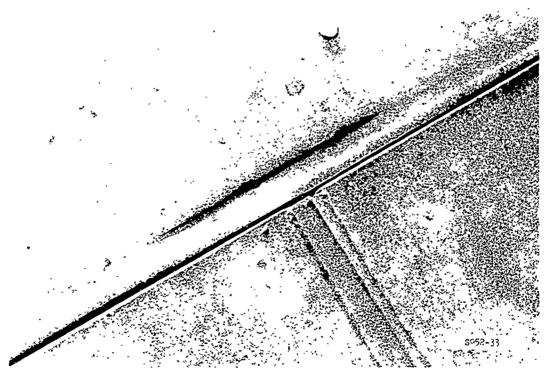
Photograph A69. Bottom lip of overlapping end joint sheared after 48 coverages



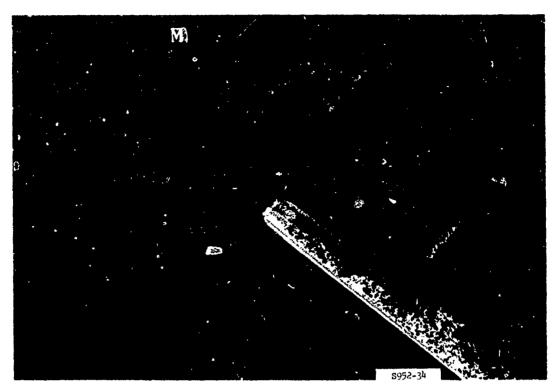
Photograph A70. Test section III, lane 2, item 3, at failure after 92 coverages



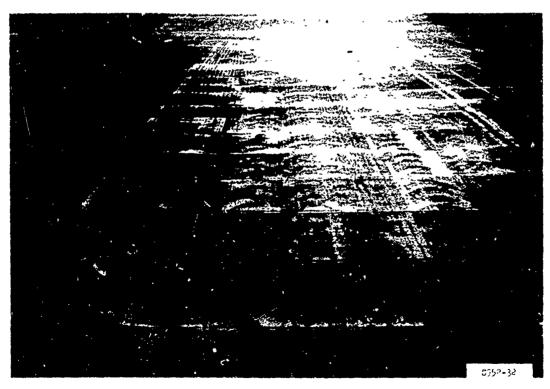
Photograph A71. Test section IV prior to traffic



Photograph A72. Top skin tear at panel center after failure at 348 coverages



Photograph A73. Top skin tear at an end joint after failure at 348 coverages



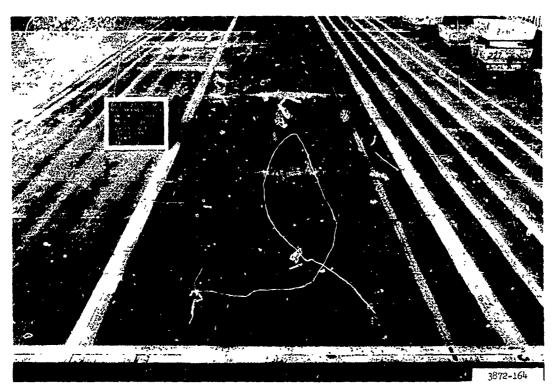
Photograph A74. Test section IV after failure at 348 coverages



Photograph A75. Test section V prior to traffic

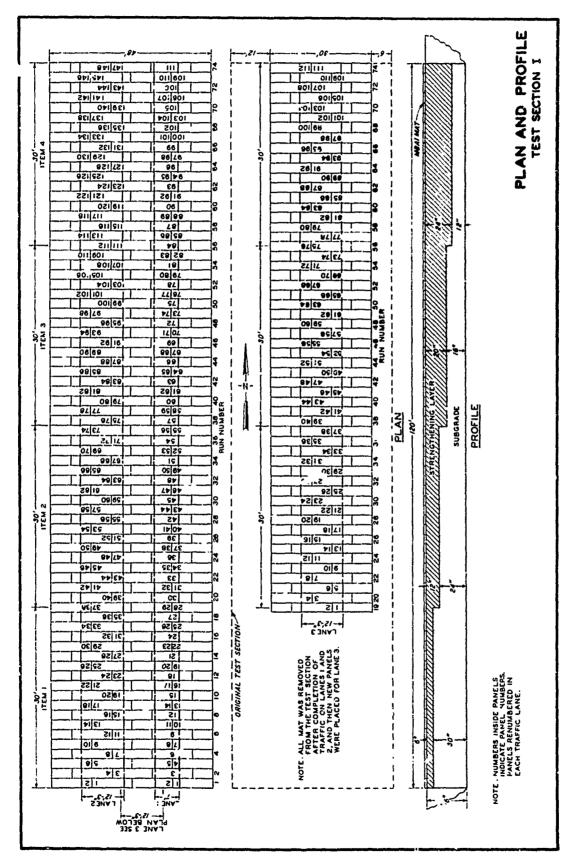


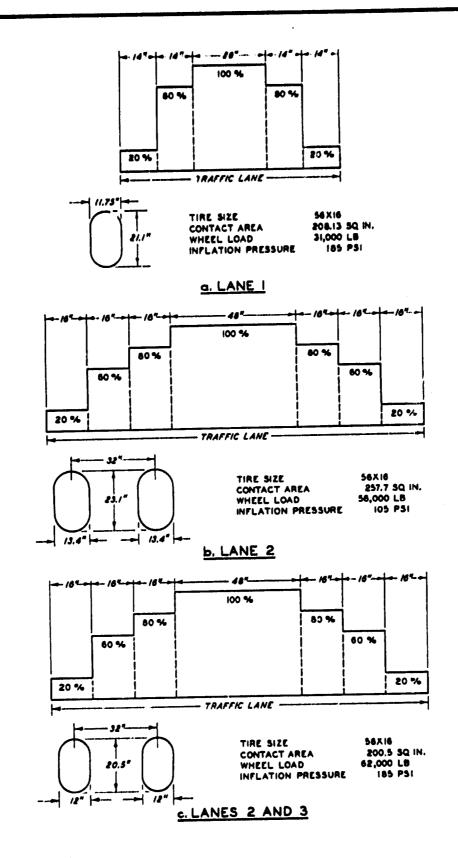
Photograph A76. Typical weld break along end joint after failure at 330 coverages



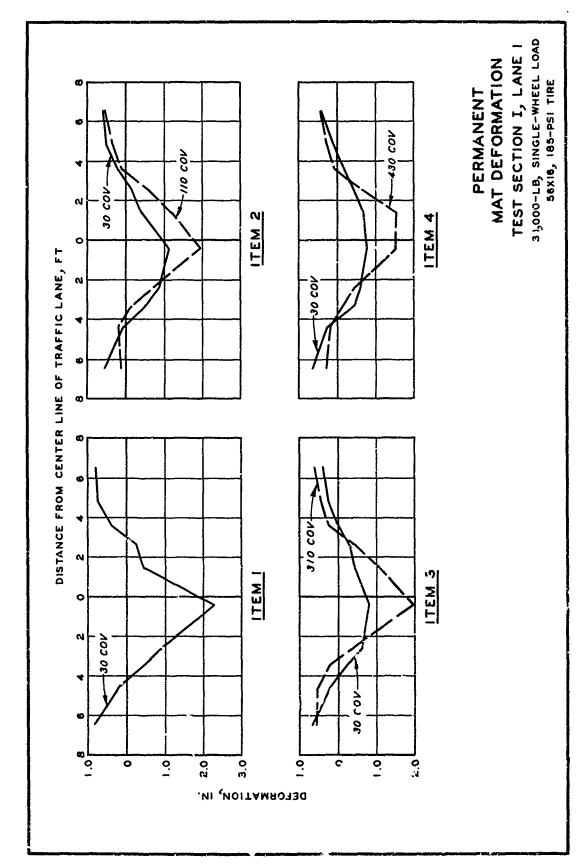
Photograph A77. Test section V after failure at 330 coverages







TRAFFIC DISTRIBUTION
PATTERNS AND TIRE
CHARACTERISTICS
TEST SECTION I



7

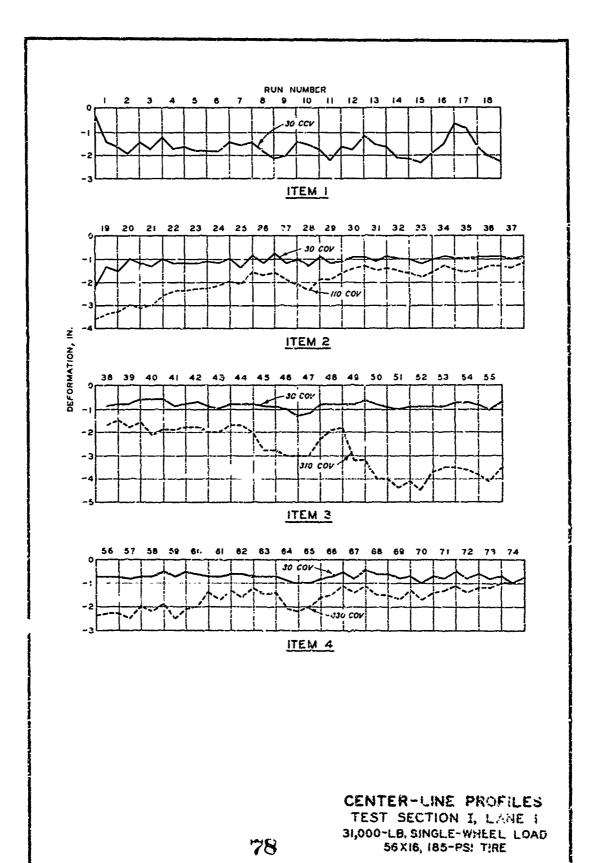
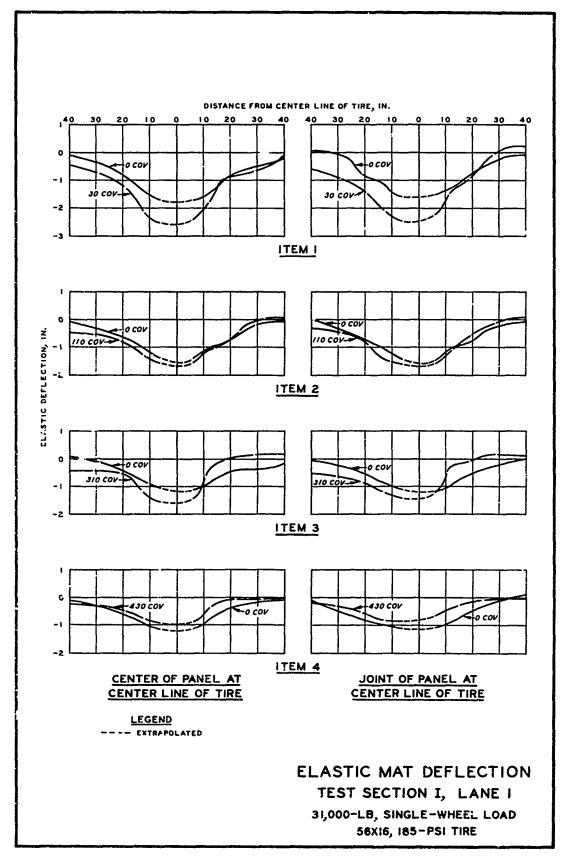


PLATE A4



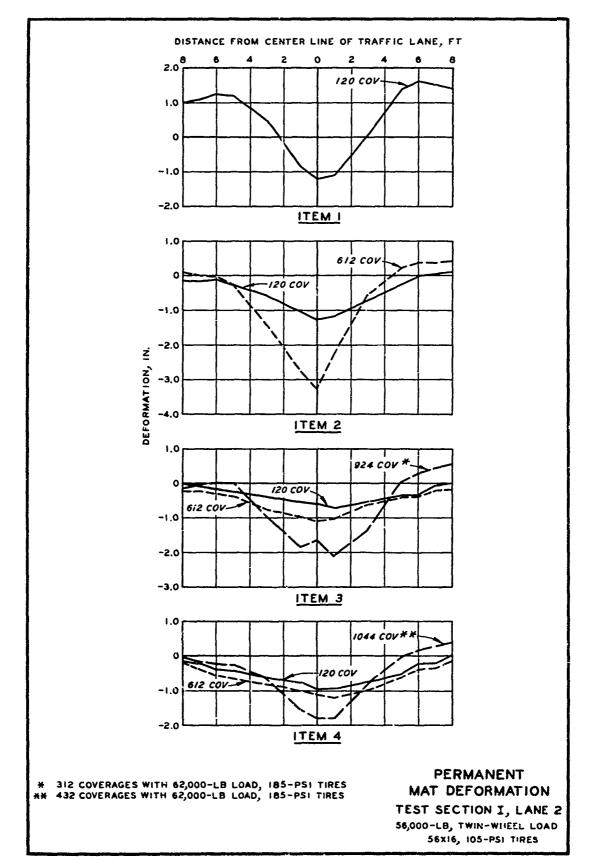
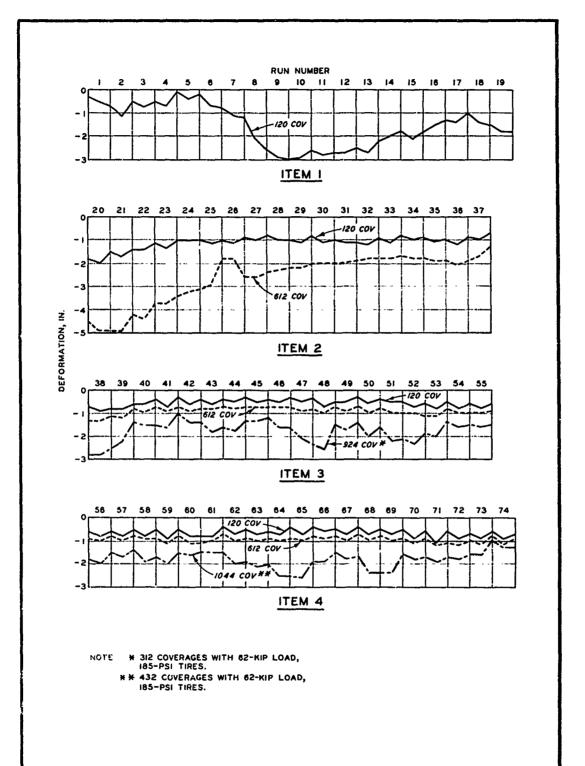
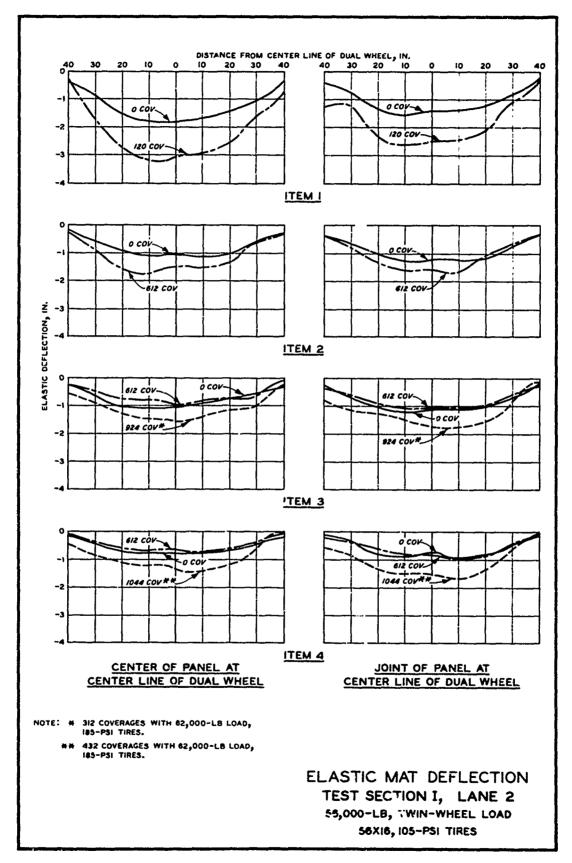
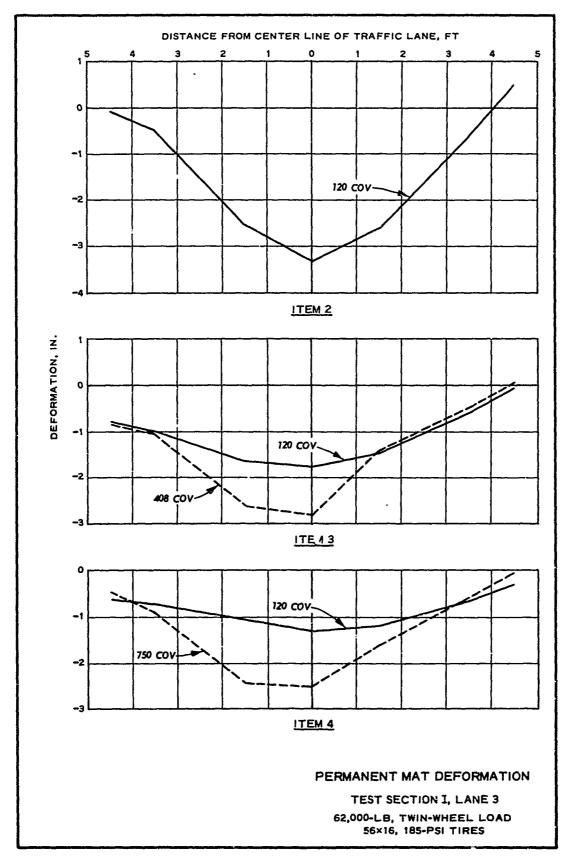


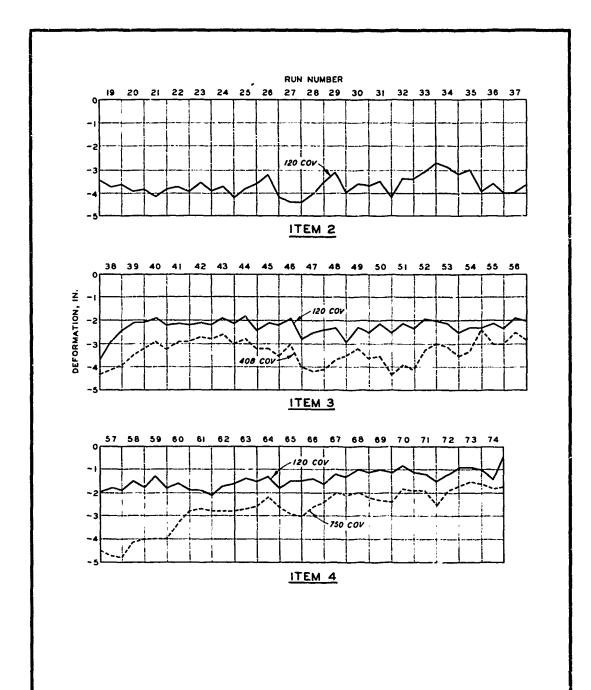
PLATE A6



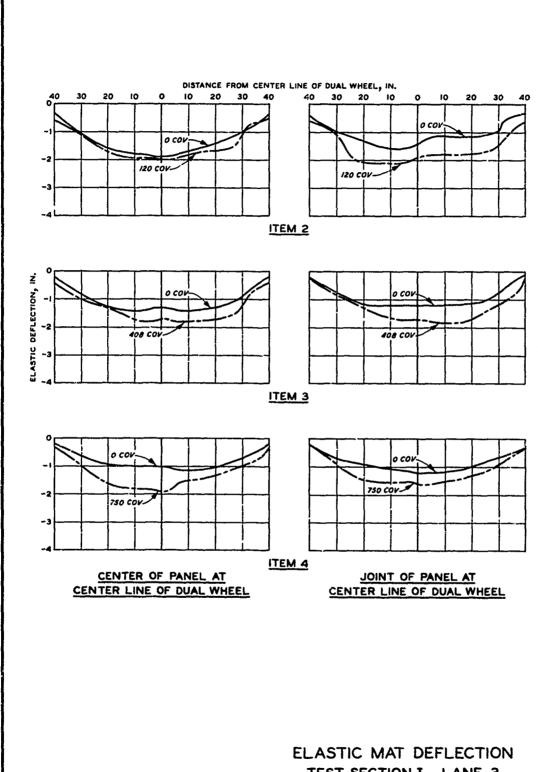
CENTER-LINE PROFILES
TEST SECTION I, LANE 2
56,000-LB, TWIN-WHEEL LOAD
56XI6, 105-PSI TIRES







CENTER-LINE PROFILES
TEST SECTION I, LANE 3
62,000-LB, TWIN-WHEEL LOAD
56X16, 185-PSI TIRES



ELASTIC MAT DEFLECTION
TEST SECTION I, LANE 3
62,000-LB, TWIN-WHEEL LOAD
56X16, 185-PSI TIRES

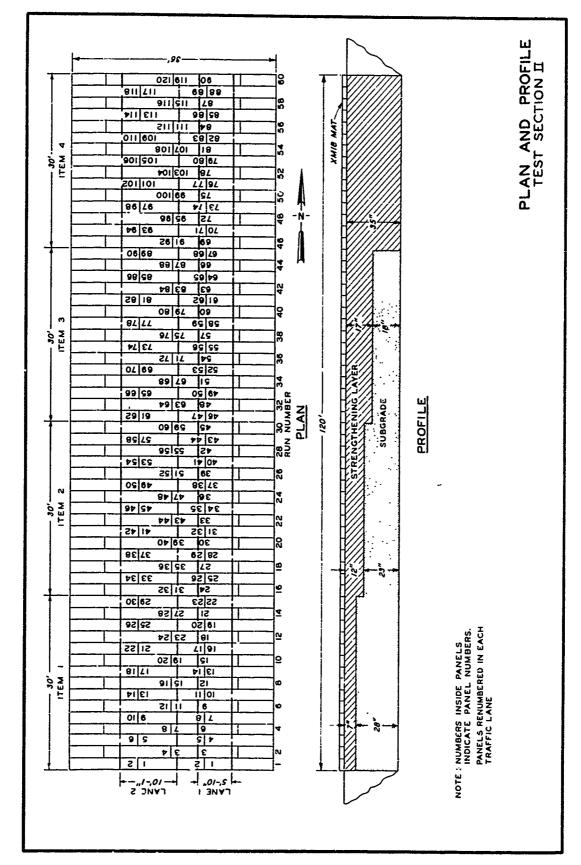
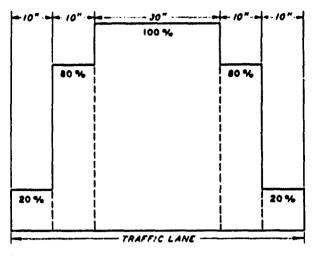


PLATE AI2



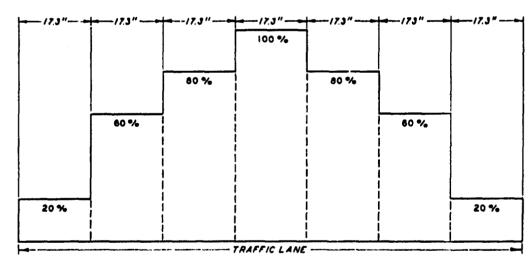


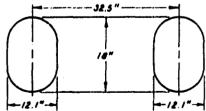
TIRE SIZE CONTACT AREA WHEEL LOAD

30X11.5 128.5 SQ IN. 30,000 LB 250 PS1

a. LANE I

INFLATION PRESSURE



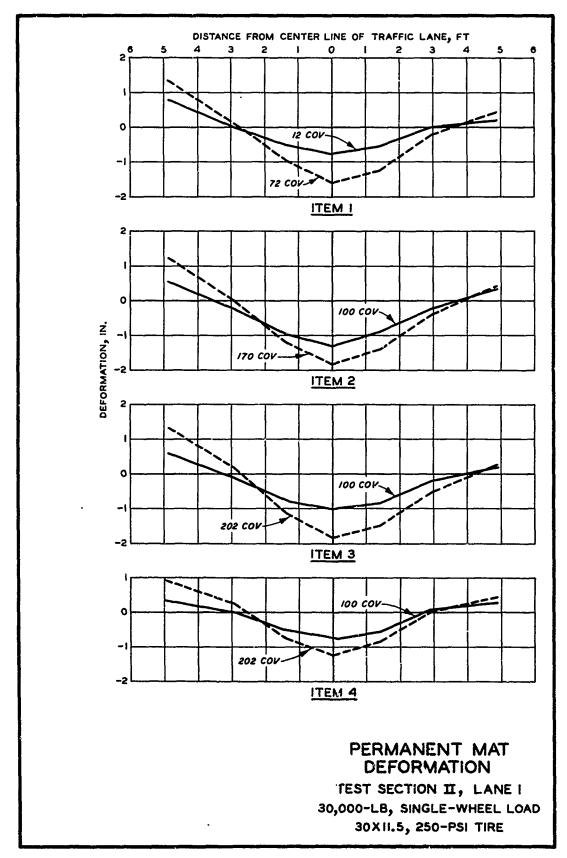


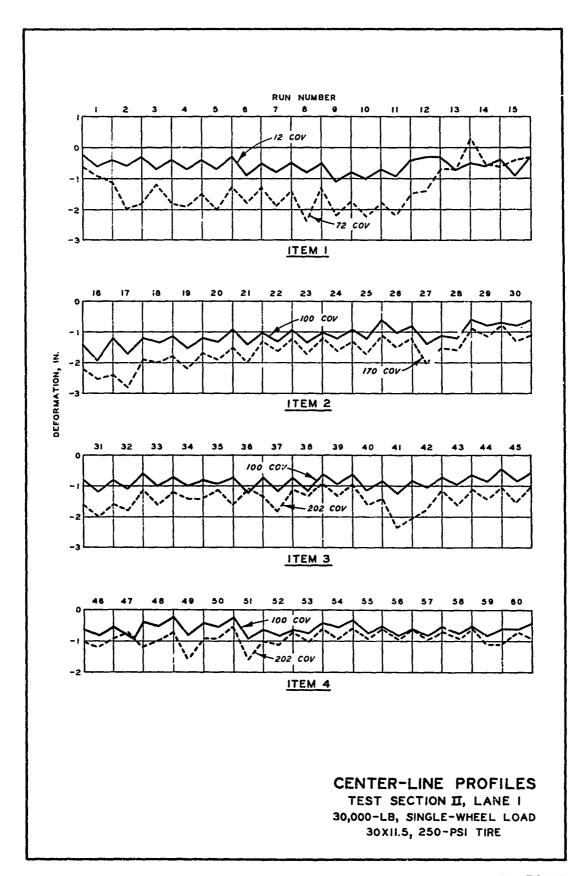
TIRE SIZE
CONTACT AREA
WHEEL LOAD
INFLATION PRESSURE

44X16 192.1 5Q IN. 70,000 LB 165 PS1

b. LANE 2

TRAFFIC DISTRIBUTION
PATTERNS AND TIRE
CHARACTERISTICS
TEST SECTION II





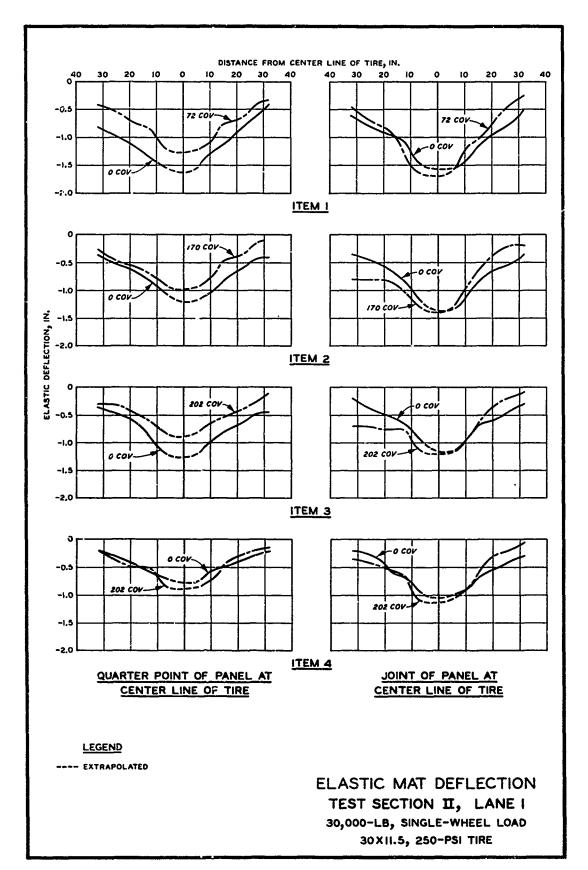
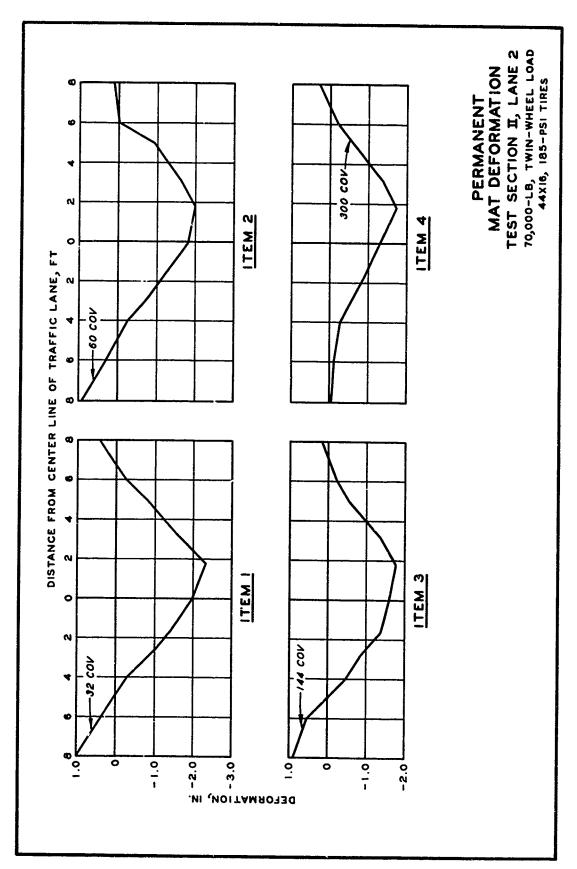
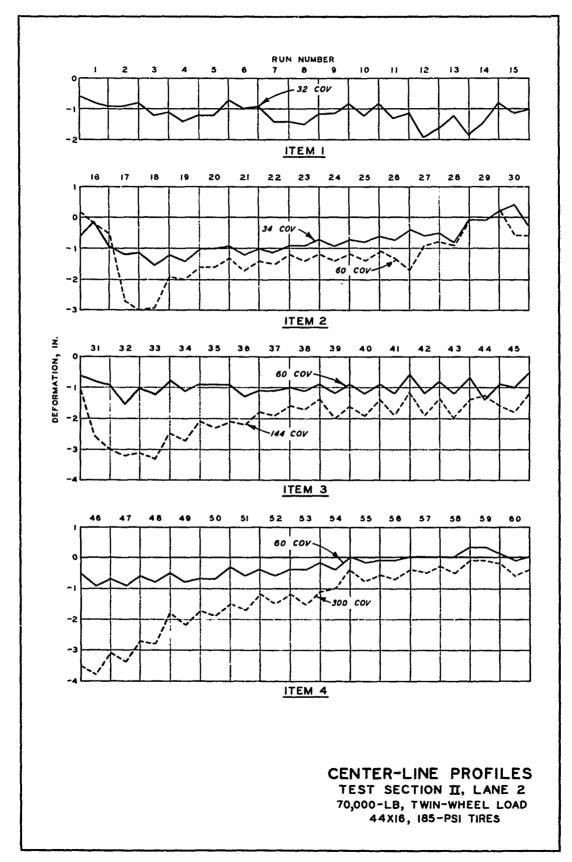
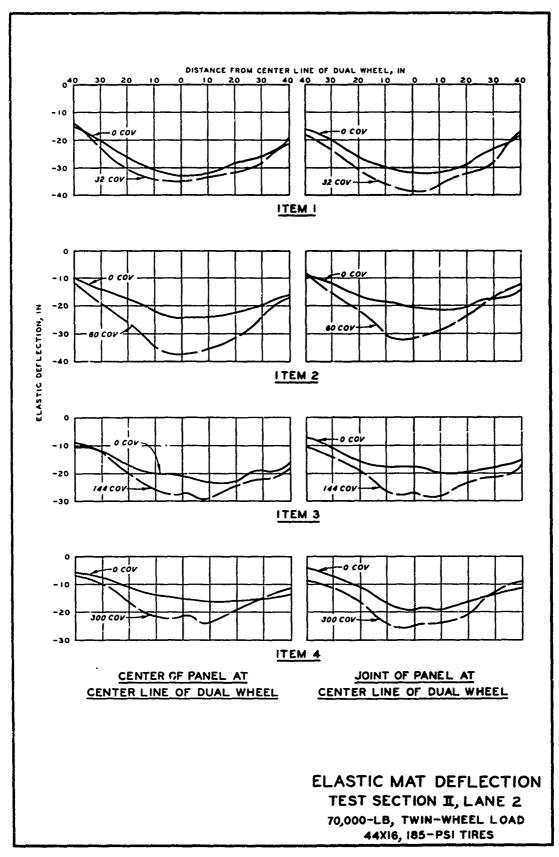


PLATE AI6



91





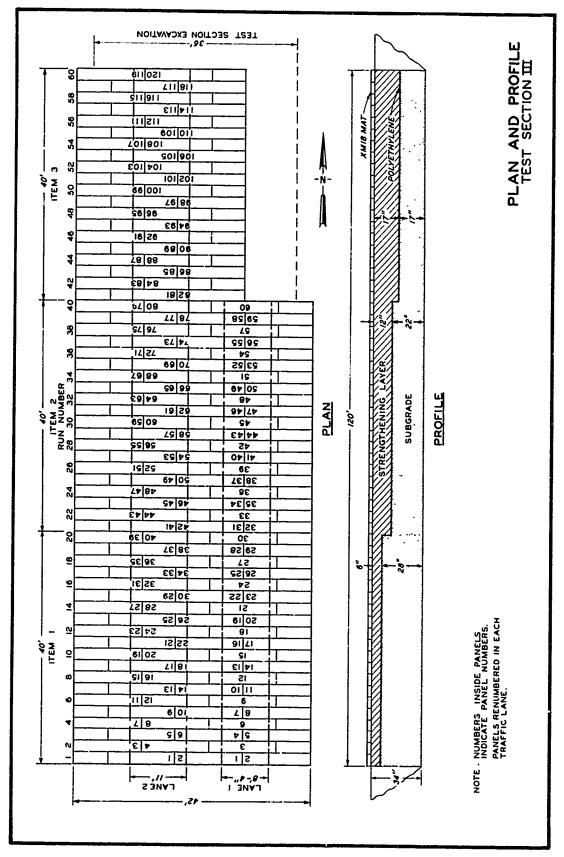


PLATE A20

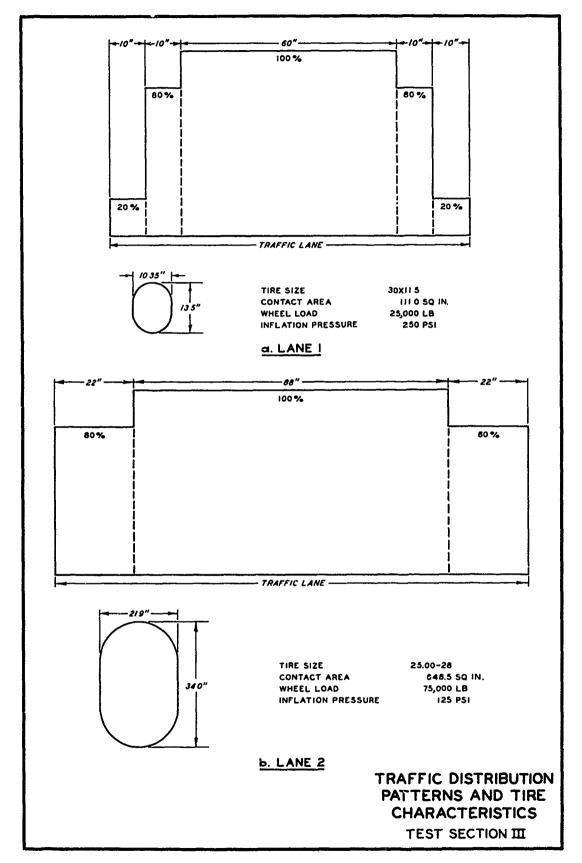


PLATE A2I

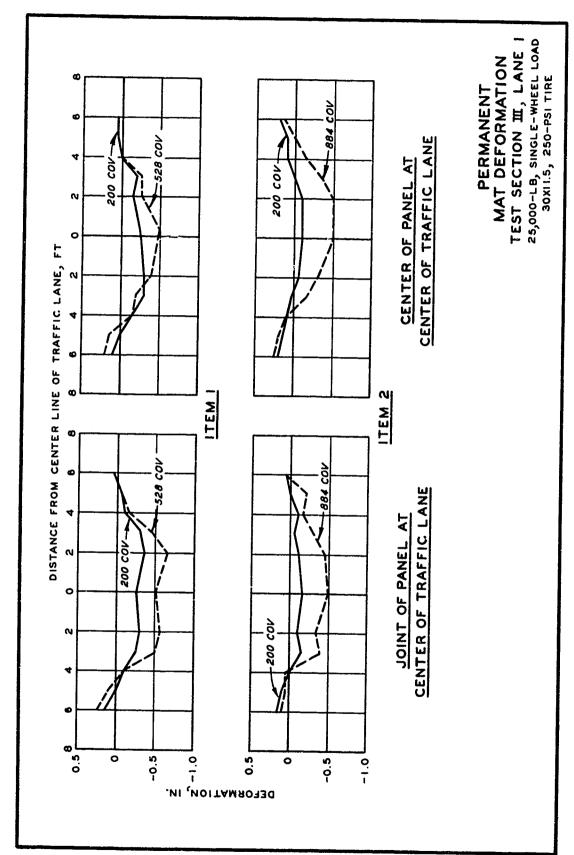
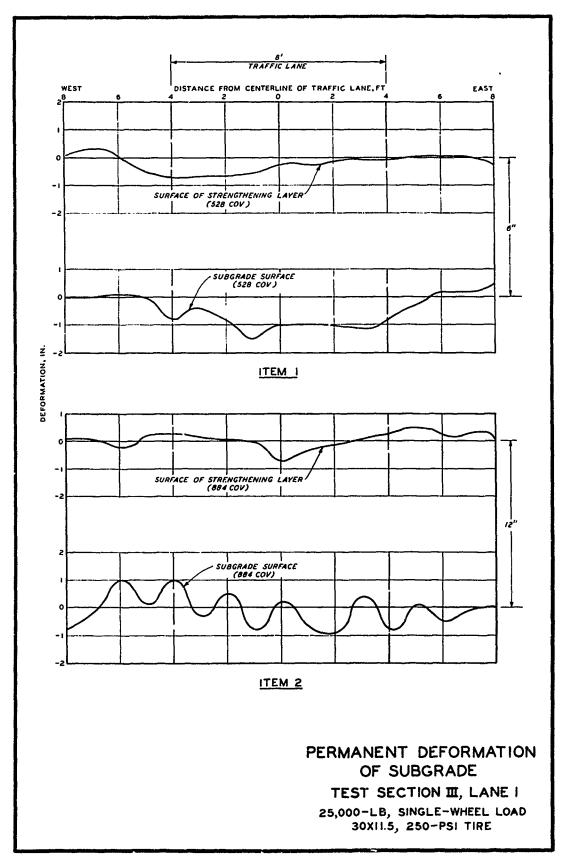
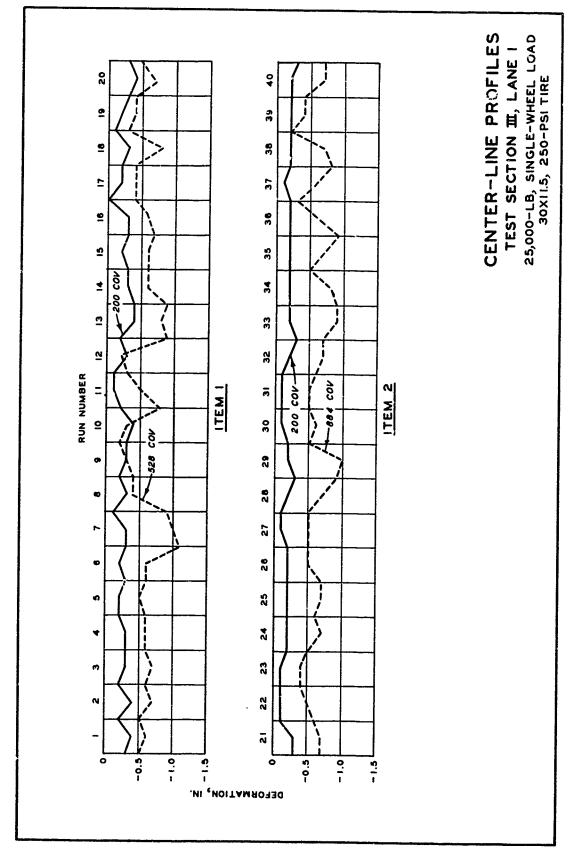
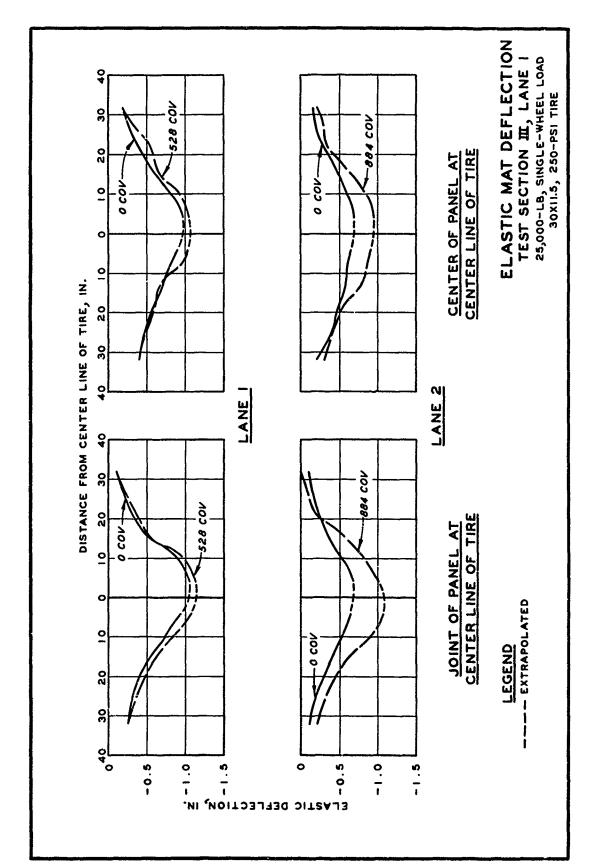


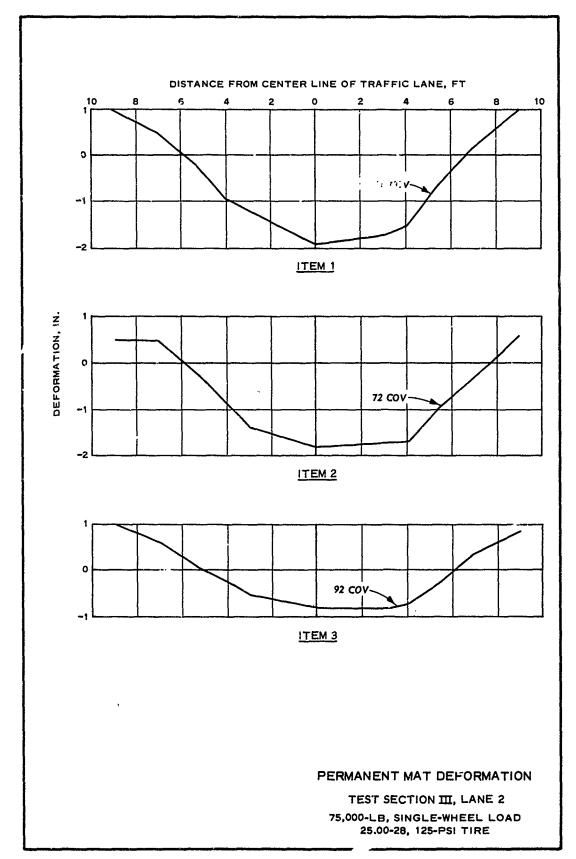
PLATE A22

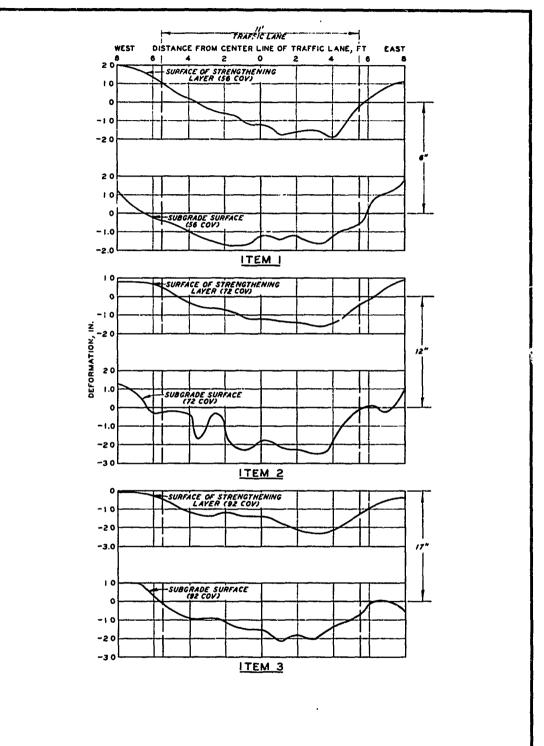






99





PERMANENT DEFORMATION OF SUBGRADE

TEST SECTION III, LANE 2

75,000-LB, SINGLE-WHEEL LOAD 25.00-28, 125-PSI TIRE

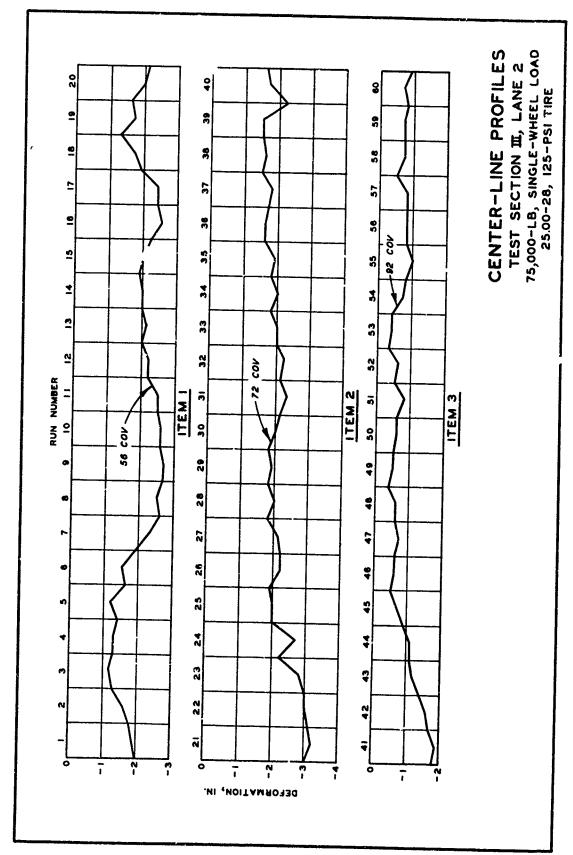
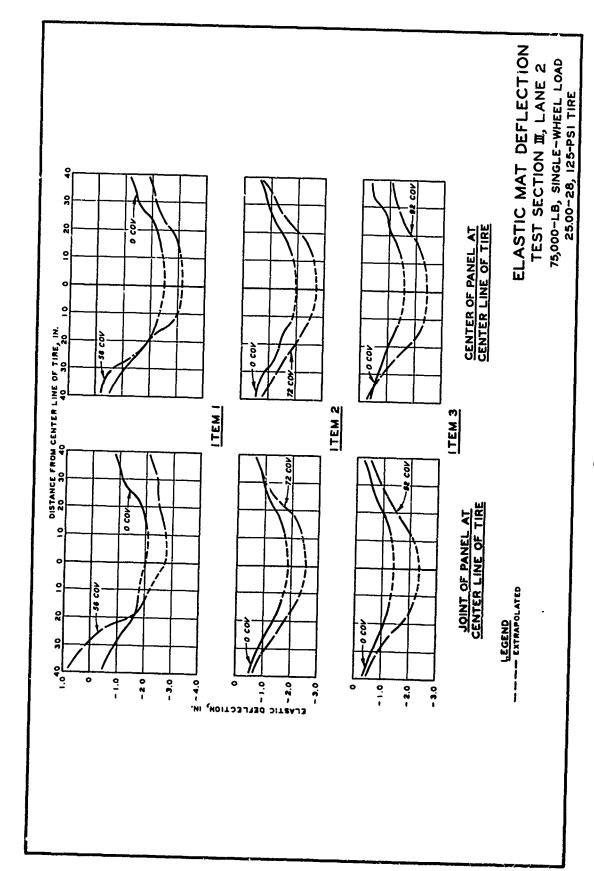
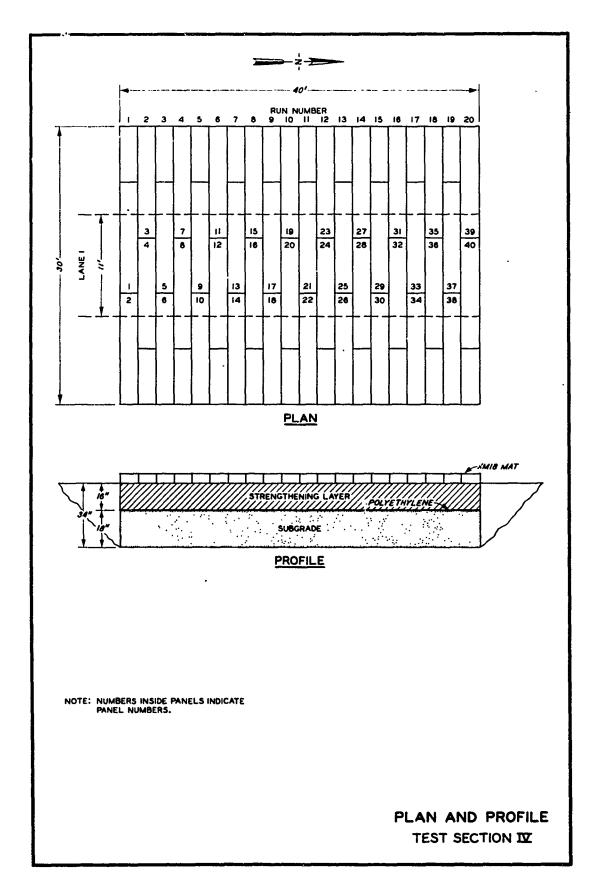


PLATE A28



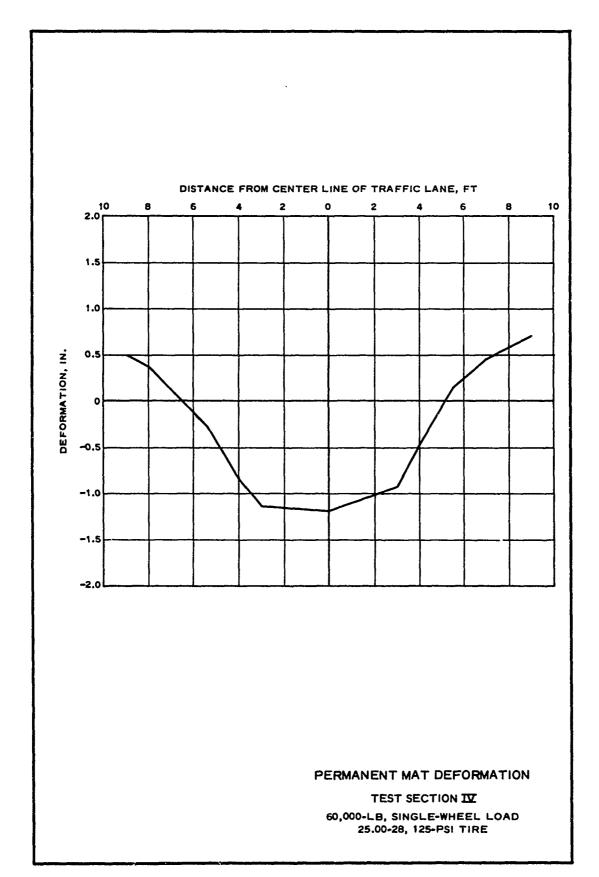
** . 5% · ×*

103



TRAFFIC DISTRIBUTION PATTERN AND TIRE PRINT CHARACTERISTICS TEST SECTION TO 80 % - 22" 538.2 SQ IN. 60,000 LB 125 PSI 25.00-28 - TRAFFIC LANE % 001 INFLATION PRESSURE CONTACT AREA WHEEL LOAD TIRE SIZE -21.1"-% 00

105



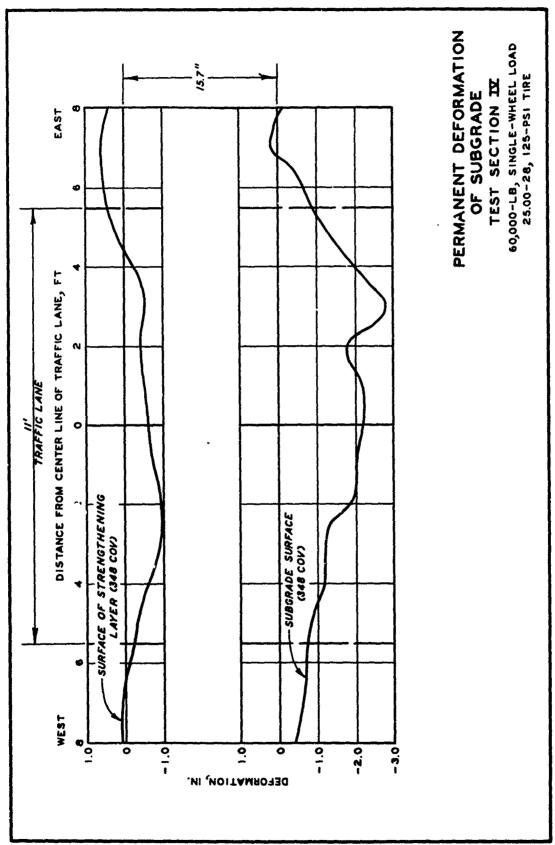


PLATE A 33

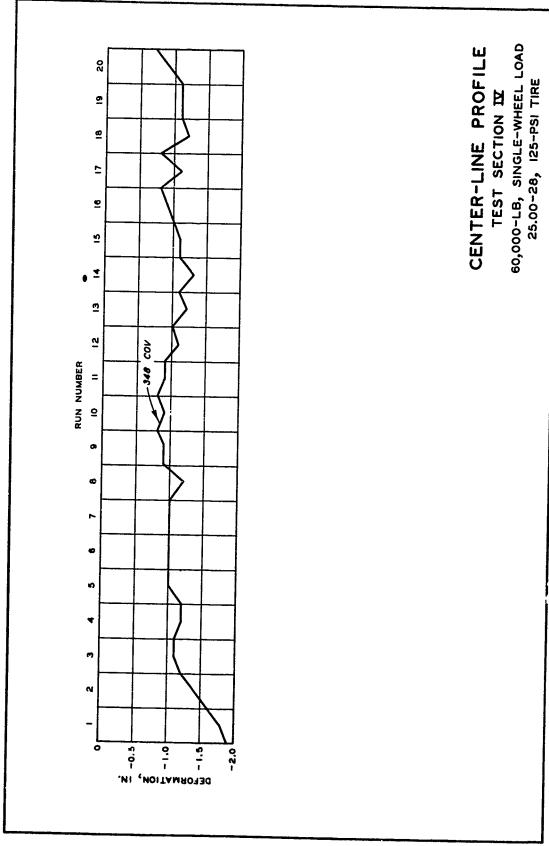
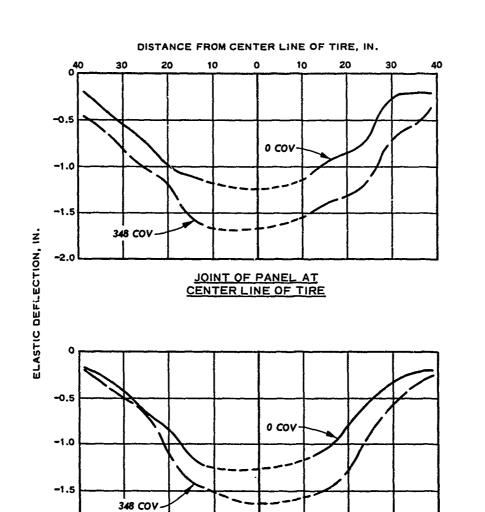


PLATE A 34



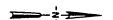
LEGEND ---- EXTRAPOLATED

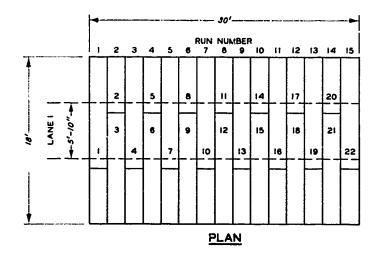
-2.0

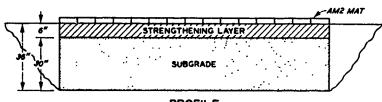
ELASTIC MAT DEFLECTION
TEST SECTION IX

60,000-LB, SINGLE-WHEEL LOAD 25.00-28. 125-PSI TIRE

CENTER OF PANEL AT CENTER LINE OF TIRE







PROFILE

NOTE: NUMBERS INSIDE PANELS INDICATE PANEL NUMBERS.

PLAN AND PROFILE TEST SECTION X

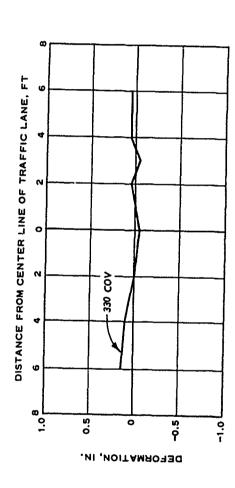
110

PLATE A 36

PATTERN AND TIRE PRINT TRAFFIC DISTRIBUTION CHARACTERISTICS TEST SECTION T 10," 20% 80% 111.0 SQ IN. 250 PSI 25,000 LB 30XII.5 - TRAFFIC LANE -30"-100% INFLATION PRESSURE CONTACT AREA WHEEL LOAD TIRE SIZE 80% 13.5" -10.35" 20%

also in a construction that extension the

111



PERMANENT MAT DEFORMATION

TEST SECTION 文 25,000-LB, SINGLE-WHEEL LOAD 30×11.5, 250-PSI TIRE

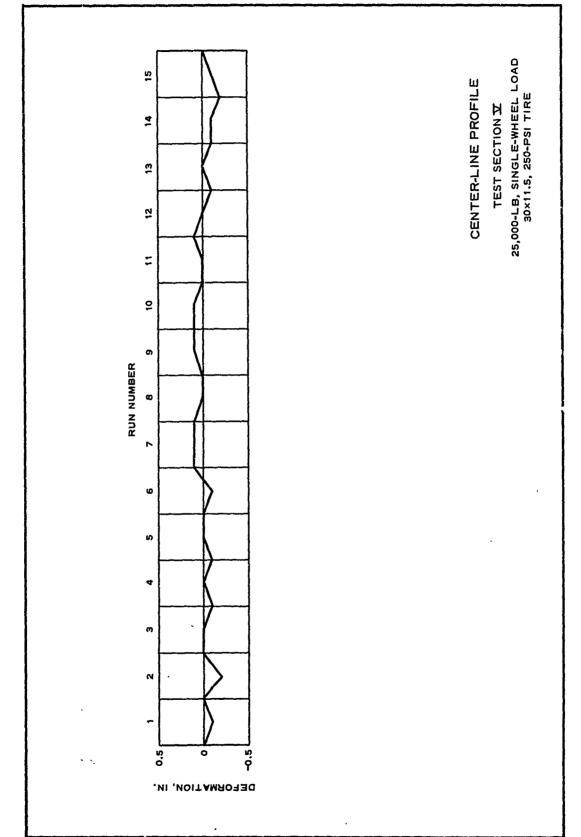
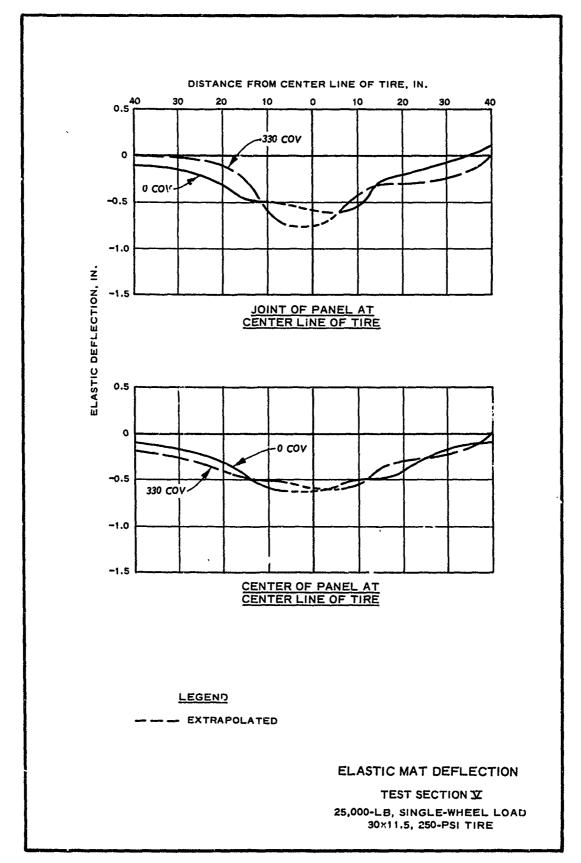


PLATE A 39



Appendix B: Thickness Reduction Curves

Thickness reduction curves for M8A1, AM2, and XM18 landing mats that were used in the analysis of test data are shown in plate B1. These curves represent the reduction in thickness (of subbase, base, and pavement) that can be applied to the pertinent flexible pavement design requirements in establishing design or evaluation criteria for the types of landing mat indicated. The curves (especially the XM18 curve) are still under study and development and thus are subject to revision.

115

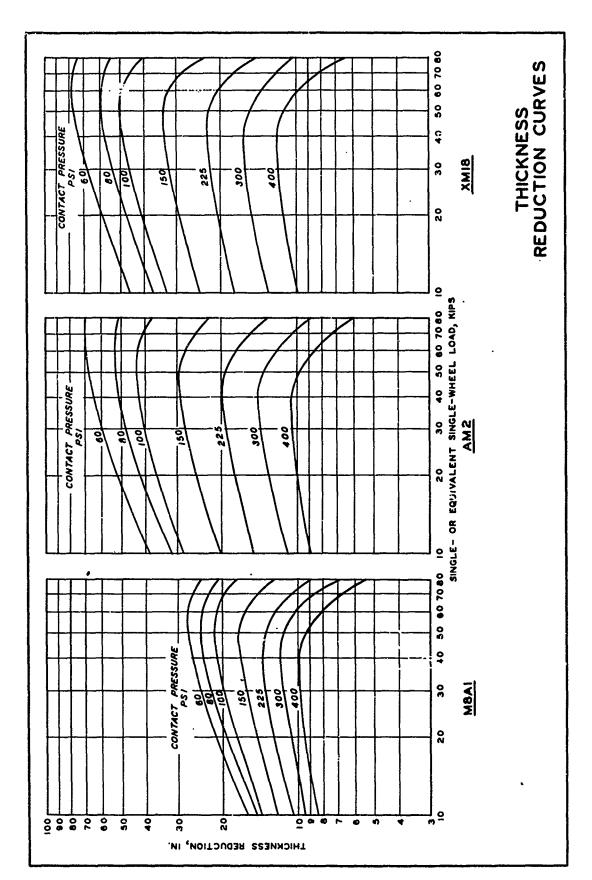


PLATE BI

Appendix C: Design Curves

As an illustration of the use of equation 2 in the main text and the thickness reduction curves in Appendix B, design curves for the C-130 and Γ -C aircraft operating from XML8 and AM2 landing mat are presented in plates Cl and C2. The curves are based on the following loadings:

		Empty	Fully Loaded
C-130	Gross weight, 1b	71,500	175,000
	Assembly load, 1b	32,175	78 , 750
	Contact area, sq in.	400	400
	Contact pressure, psi	40	98
Fl4-C	Gross weight, lb	28,539	59,064
	Assembly load, 1b	12,843	26,57 9
	Contact area, sq in.	100	100
	Contact pressure, psi	128	2 66

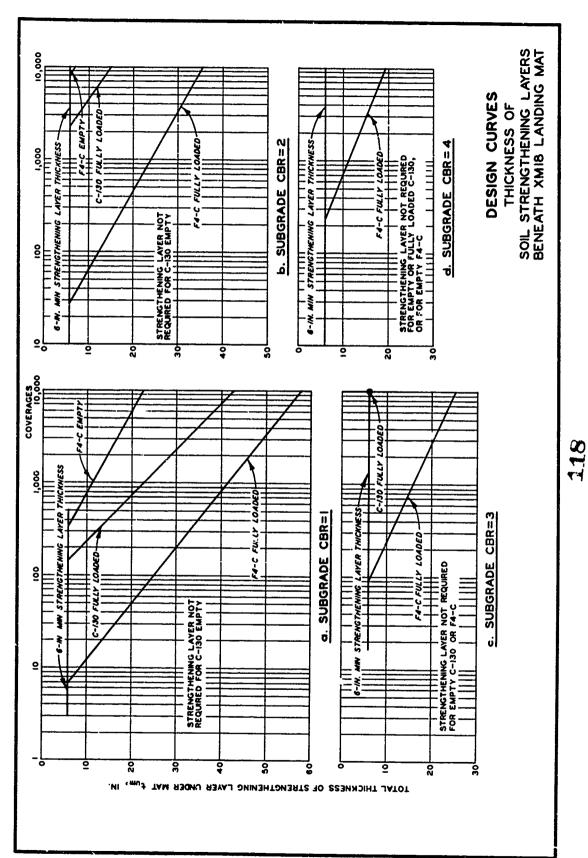


PLATE CI

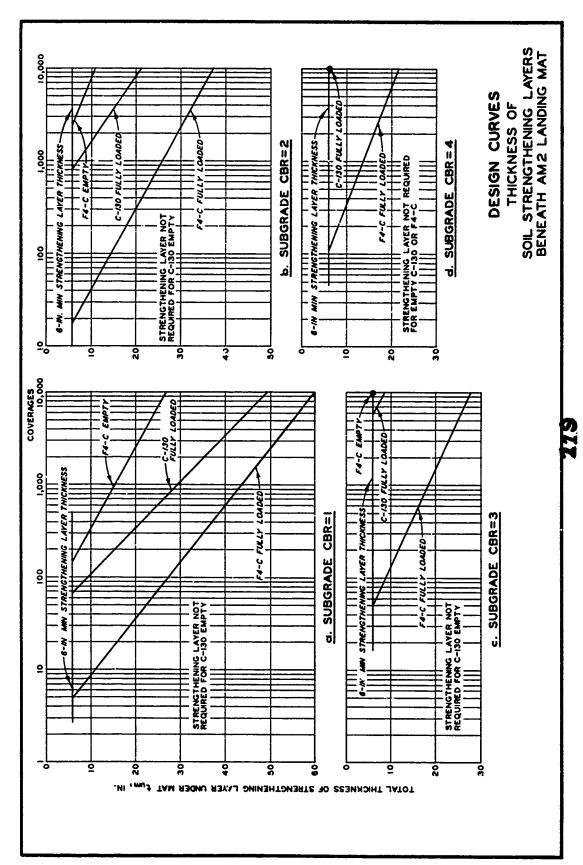


PLATE C2